

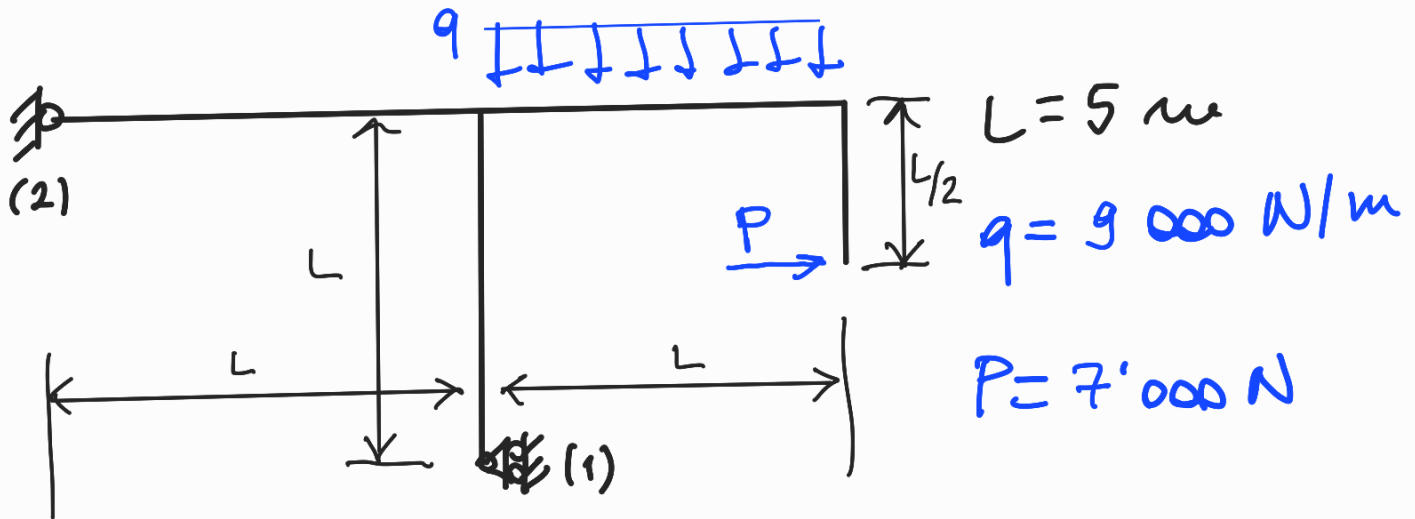
1) • Verifica che la struttura è isostatica

$$n_{aste} = 1$$

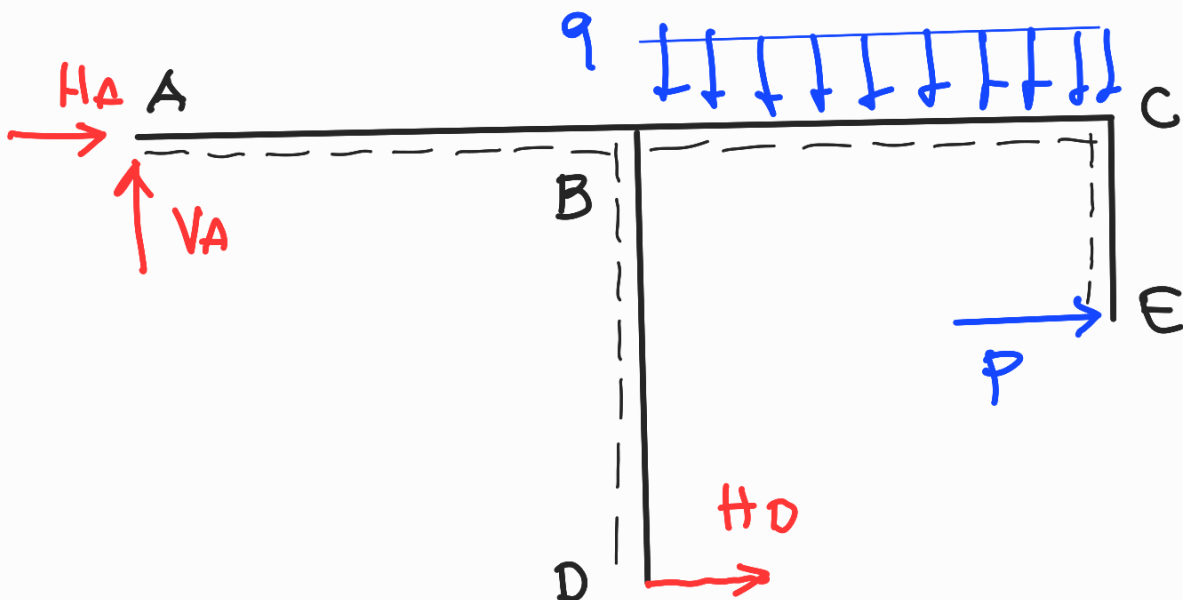
$$v = 2 + 1 = 3$$

$$g = 3$$

→ isostatica



• Calcolo delle reazioni vincolari



Equilibri delle aste

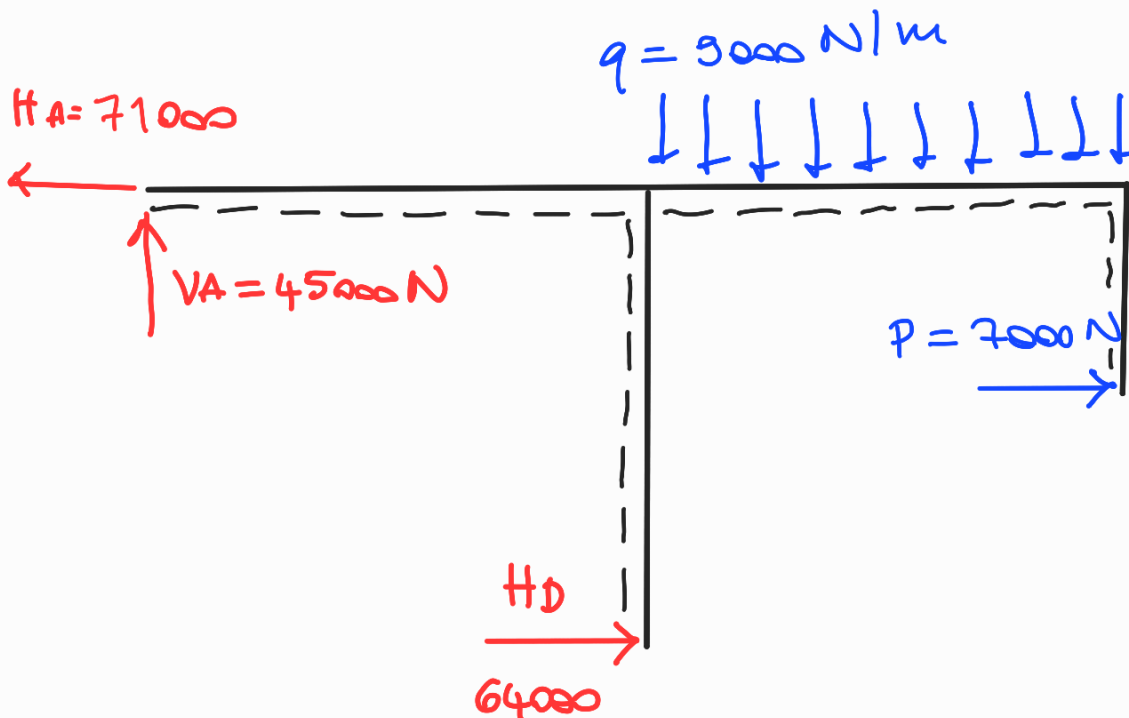
$$\left\{ \begin{array}{l} \rightarrow HA + HD + P = 0 \\ \uparrow VA - qL = 0 \\ \curvearrowright_A qL \frac{3}{2}L - HD \cdot L - P \frac{L}{2} = 0 \end{array} \right.$$

$$VA = qL = 45000 \text{ N}$$

$$HD = \frac{3}{2}qL - \frac{P}{2} = 64000 \text{ N}$$

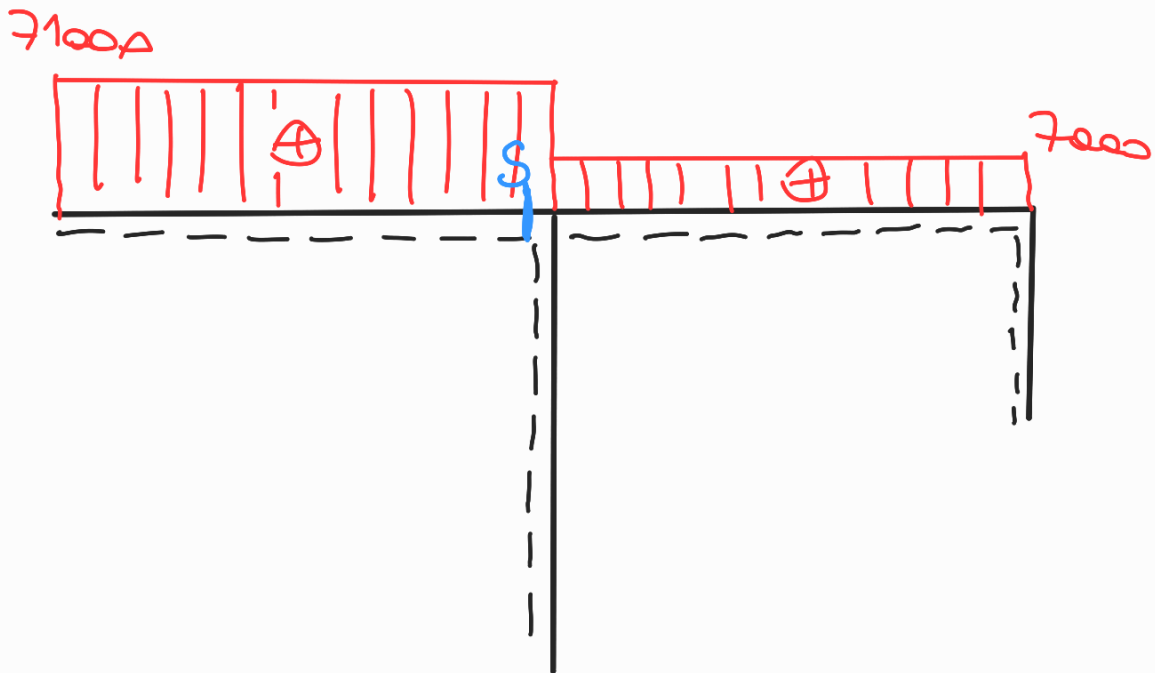
$$HA = -P - HD = 71000 \text{ N}$$

Schema finale delle forze attive e reattive

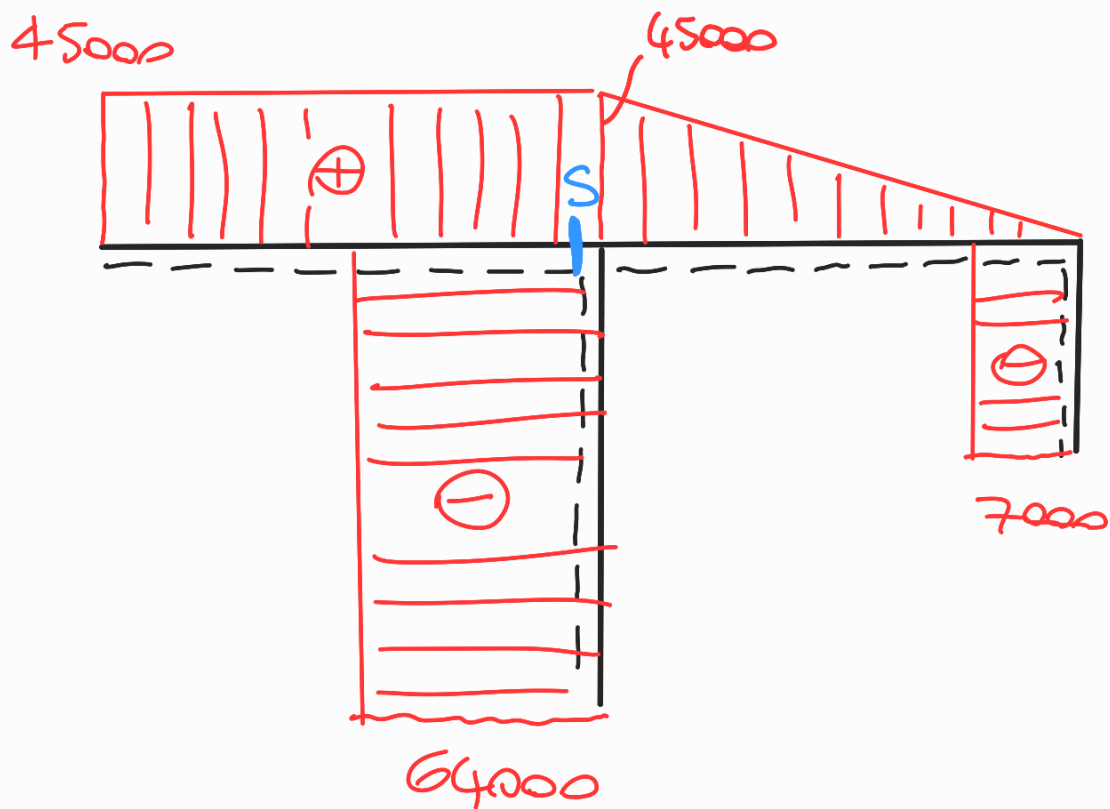


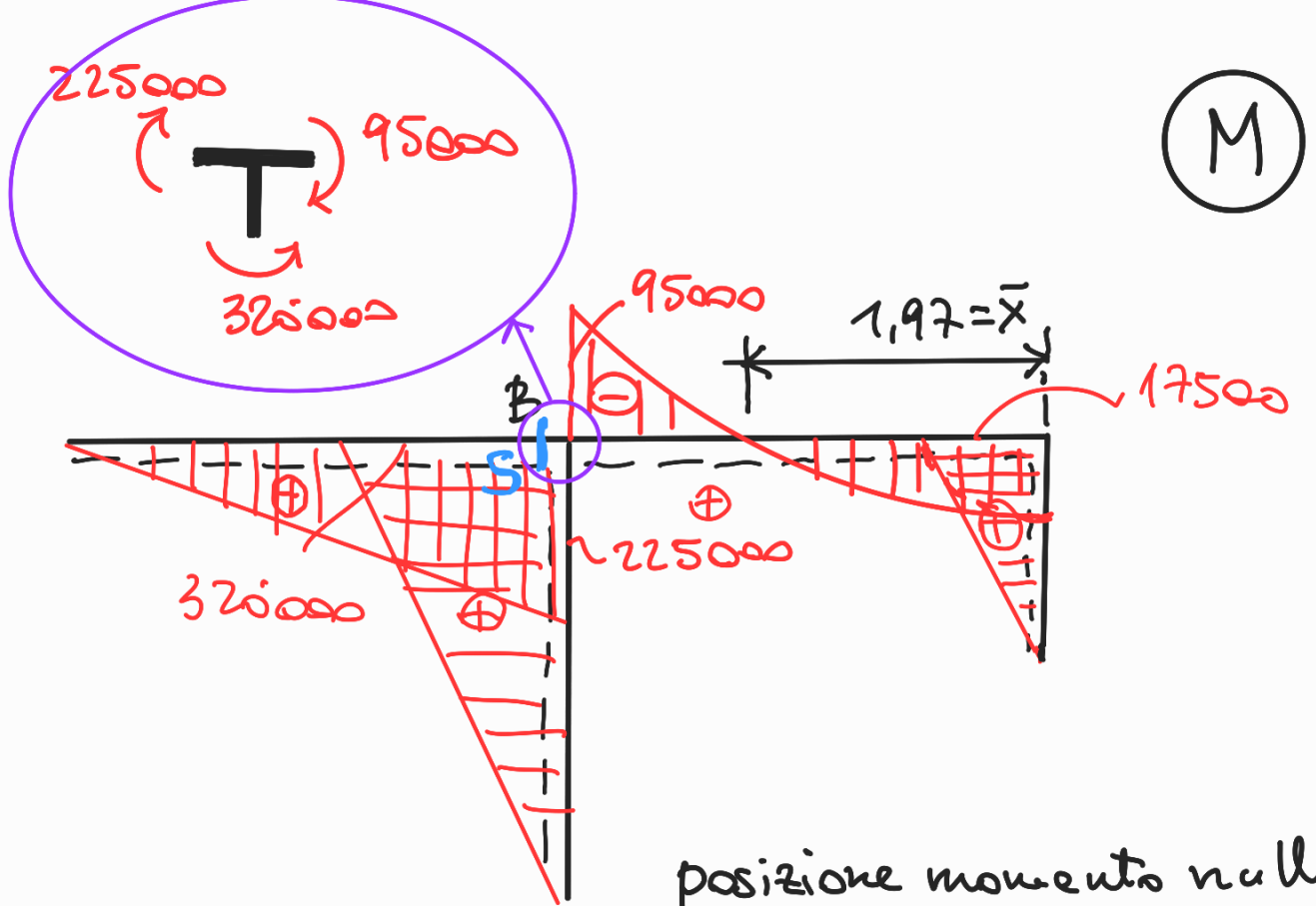
• Diagrammi delle azioni interne

(2)



(T)



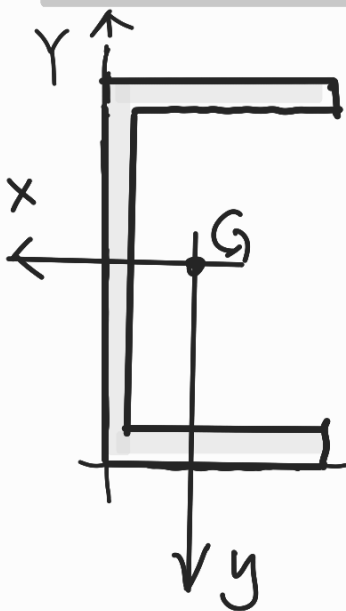


posizione momento nullo:

$$9 \frac{\bar{x}^2}{2} - P \cdot \frac{L}{2} = 0 \rightarrow \bar{x} = 1,97$$

2)

● Caratteristiche geometriche della sezione



$$A = 2(0,15 \cdot 0,01) + 0,3 \cdot 0,01 = 0,006 \text{ m}^2$$

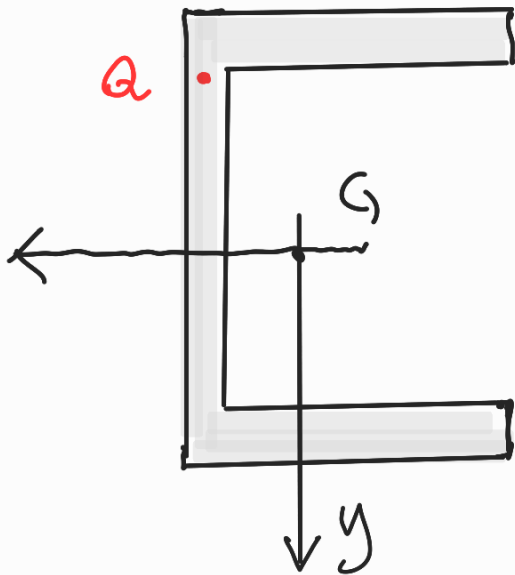
Posizione baricentro

$$\begin{cases} Y_G = 0,15 \\ X_G = \frac{S_y}{A} = \frac{0,15 \cdot 0,01 \cdot 0,075 \cdot 2}{0,006} = \\ = 0,0375 \text{ m} \end{cases}$$

$$I_x = \frac{0,01 \cdot 0,3^3}{12} + \left(\frac{0,15 \cdot 0,01^3}{12} + 0,15 \cdot 0,01 \cdot 0,075^2 \right) \cdot 2 \approx 9 \cdot 10^{-5} \text{ m}^4$$

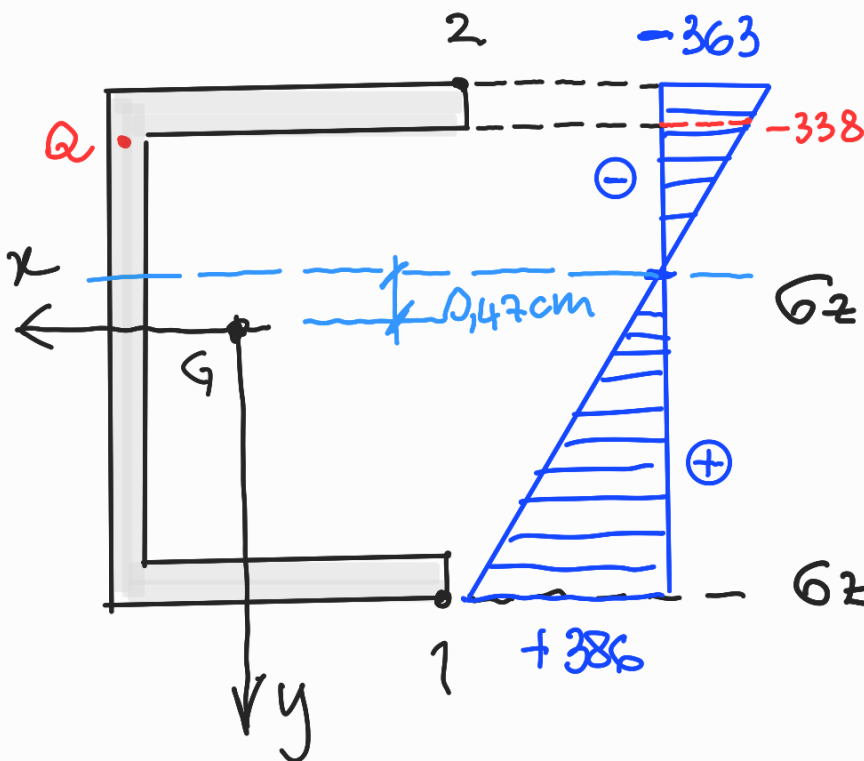
Il momento di inerzia I_y non è necessario.

● Calcolo delle Tensioni nelle sez. S-S



$$\left\{ \begin{array}{l} N = +71000 \text{ N} \\ T = 45000 \text{ N} \\ M_x = 225000 \text{ Nm} \end{array} \right.$$

● Pressoflessione



$$\sigma_z(y) = \frac{N}{A} + \frac{M_x y}{I_x}$$

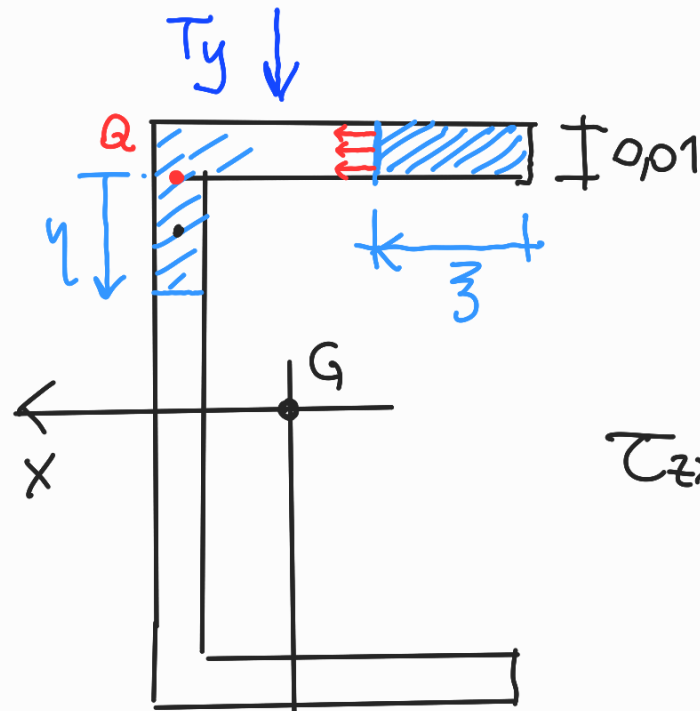
$$\begin{aligned} \sigma_z(1) &= \frac{71000}{6 \cdot 10^{-3}} + \frac{225000}{9 \cdot 10^{-5}} \cdot 0,15 = \\ &= 386 \text{ MPa} \end{aligned}$$

$$\begin{aligned} \sigma_z(2) &= \frac{71000}{6 \cdot 10^{-3}} + \frac{225000}{9 \cdot 10^{-5}} (-0,15) = \\ &= -363 \text{ MPa} \end{aligned}$$

$$\sigma_z(Q) = \frac{71000}{6 \cdot 10^{-3}} + \frac{225000(-0,14)}{9 \cdot 10^{-5}} = -338 \text{ MPa}$$

• Taglio (f. di Jourawski)

$$\tau_{zs} = - \frac{T_y S_x^*}{I_x \cdot b}$$



Si assume che il taglio T_y passi per il centro di taglio

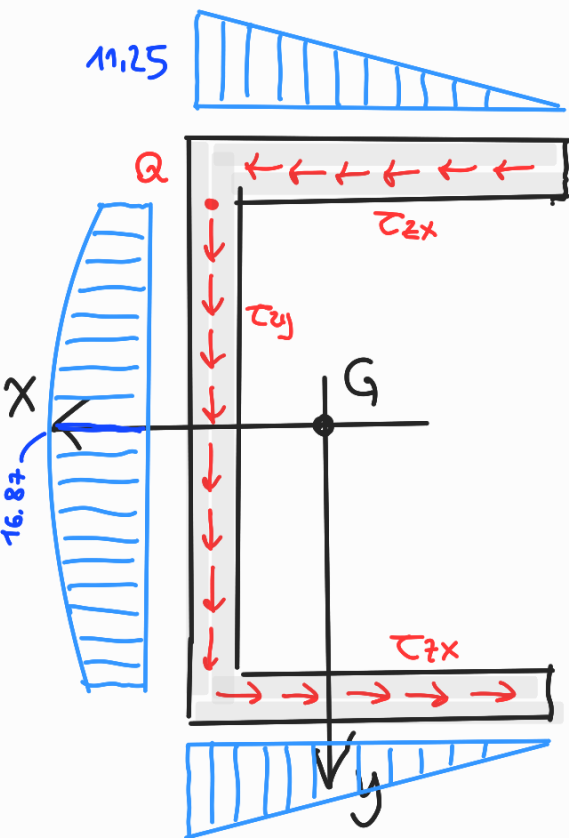
$$\tau_{zx}(z) = \frac{-45000 \cdot (0,01 \cdot z \cdot (-0,15))}{9 \cdot 10^{-5} \cdot 0,01}$$

$$\tau_{zx}(z=0,15) = \frac{-45000 (0,01 \cdot 0,15 (-0,15))}{9 \cdot 10^{-5} \cdot 0,01} = 11,25 \text{ MPa}$$

$\tau_{zx}(Q) = 11,25 \text{ MPa}$

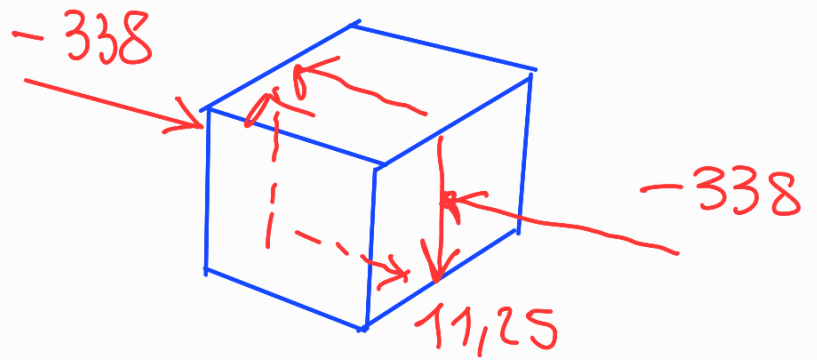
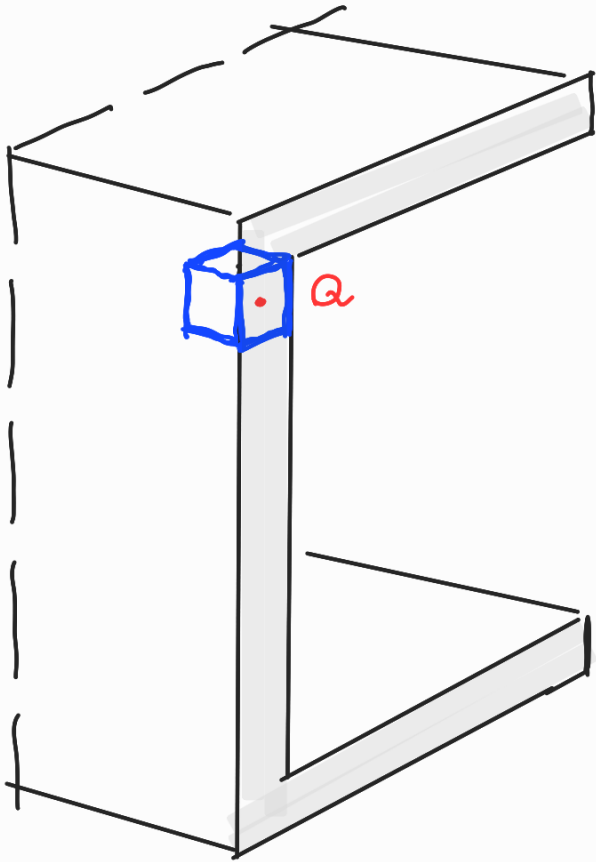
$$\tau_{zy}(\eta) = \frac{-45000 \cdot [-0,15 \cdot 0,01 \cdot 0,15 - 0,01 \cdot \eta (0,15 - \frac{\eta}{2})]}{9 \cdot 10^{-5} \cdot 0,01}$$

$$\tau_{zy}^{\max} = \tau_{zy}(\eta=0,15) = 16,87 \text{ MPa}$$

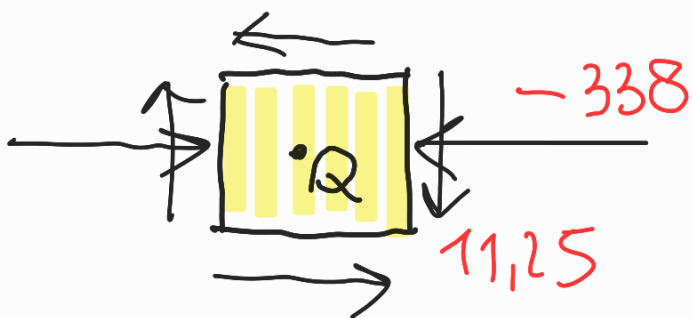


Distribuzione delle tensioni tangenziali nella sezione

- Verifica di resistenza nel punto Q della set S
e cerchio di Mohr



Punto Q (MPa) stato tensionale

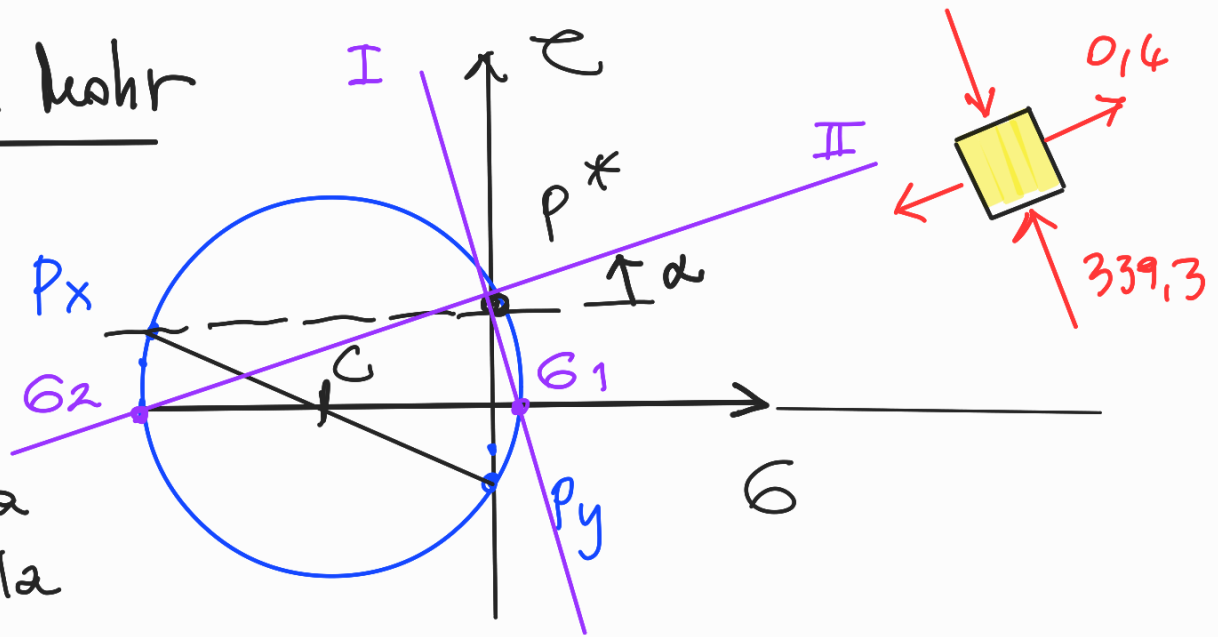


$$P_x = (-338 ; 11,25)$$

$$P_y = (0 ; -11,25)$$

Cerchio di Mohr

Diagramma non in scala



$$\sigma_c = \frac{-338 + 0}{2} = -169,5 \text{ MPa}$$

$$r = 169,8 \text{ MPa}$$

$$\begin{cases} \sigma_1 = \sigma_c + r = 0,4 \text{ MPa} & \text{Tensioni} \\ \sigma_2 = \sigma_c - r = -339,3 \text{ MPa} & \text{principali} \end{cases}$$

$$\alpha = \frac{1}{2} \arctg \frac{\tau_{xy}}{\frac{\sigma_x - \sigma_y}{2}} = 3,8^\circ \text{ antiorario}$$

• Verifica di resistenza nel punto Q

Criterio di Von Mises

$$\sigma_{eqVM} = \sqrt{\sigma^2 + 3\tau^2}$$

$$G_{eqvH} = \sqrt{338^2 + 3 \cdot 11,25^2} = 338,6 \text{ MPa}$$

$$\underline{G_{eqvH}} = \underline{338,6 \text{ MPa}} < 400 \text{ MPa}$$

verificato
