

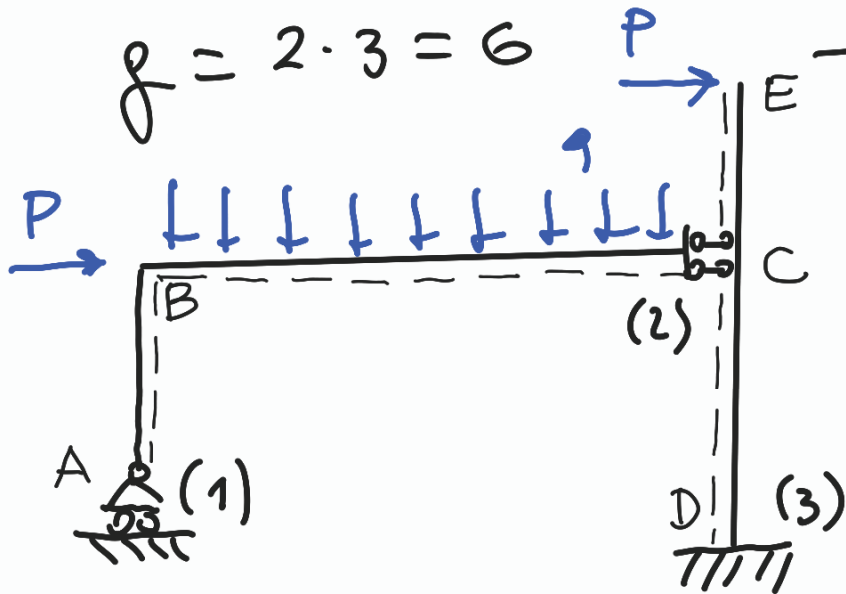
1) • Verifica che la struttura è isostatica

$n_{aste} = 2$

$v = 3 + 2 + 1 = 6$

$f = 2 \cdot 3 = 6$

$P \rightarrow$ isostatica

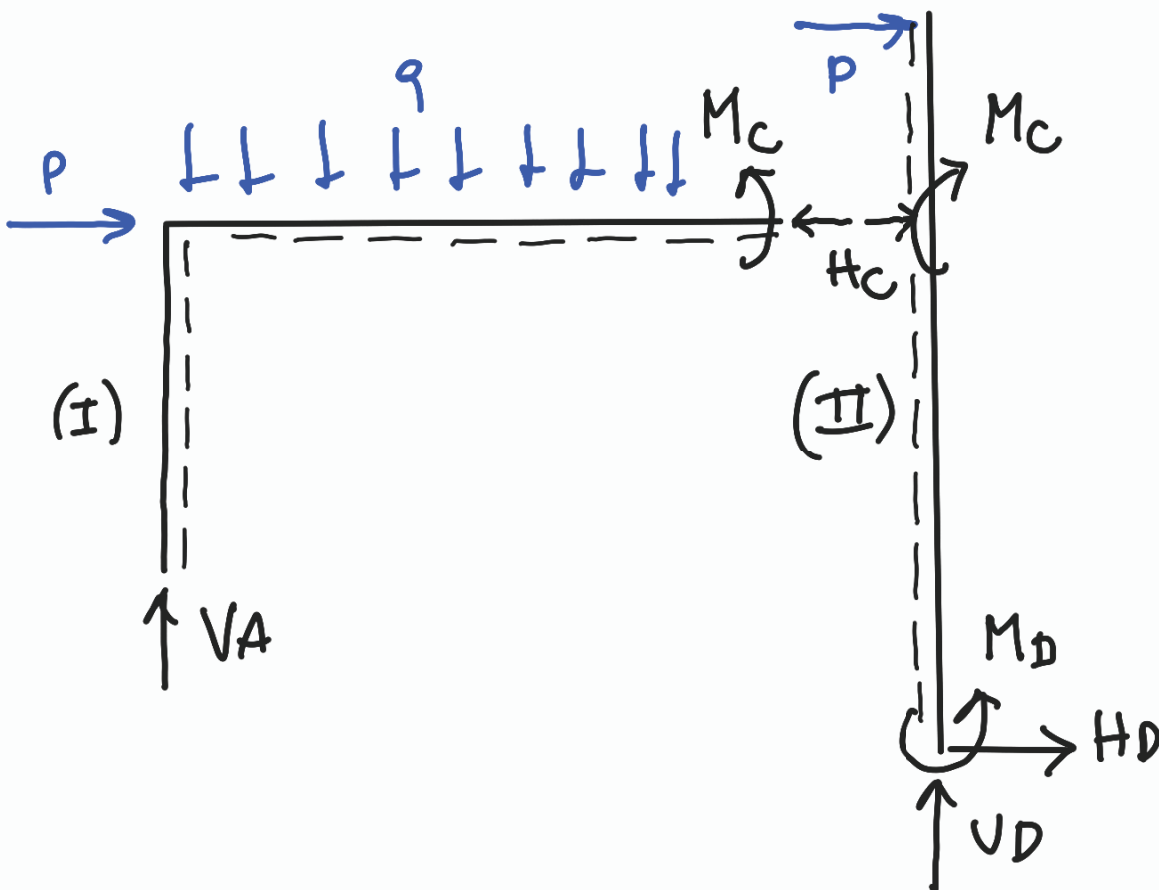


$L = 5 \text{ m}$

$q = 6000 \text{ N/m}$

$P = 5000 \text{ N}$

• Calcolo delle reazioni vincolari



For. libere delle aste:

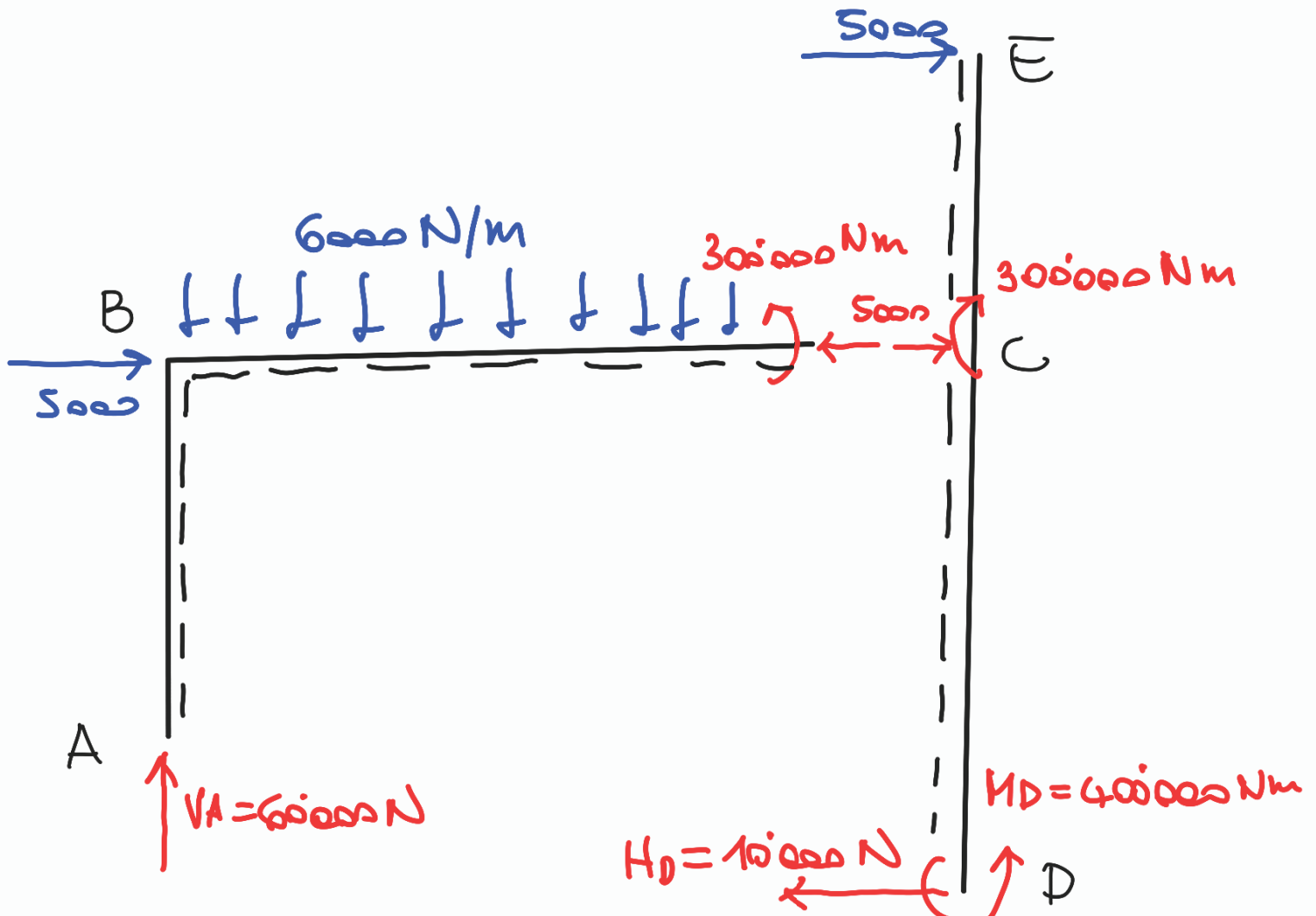
$$I \begin{cases} \rightarrow P - H_C = 0 & H_C = P = 50000 \text{ N} \\ \uparrow V_A - q \cdot 2L = 0 & V_A = 2qL = 60000 \text{ N} \\ \curvearrowright_A PL + q \cdot 2L \cdot L - H_C \cdot L = 0 \end{cases}$$

$$II \begin{cases} \rightarrow P + H_C + H_D = 0 & H_D = -P - H_C = -100000 \text{ N} \\ \uparrow V_D = 0 & V_D = 0 \\ \curvearrowright_D -M_D + H_C \cdot \frac{3}{2}L + P \cdot (L + \frac{3}{2}L) = 0 \end{cases}$$

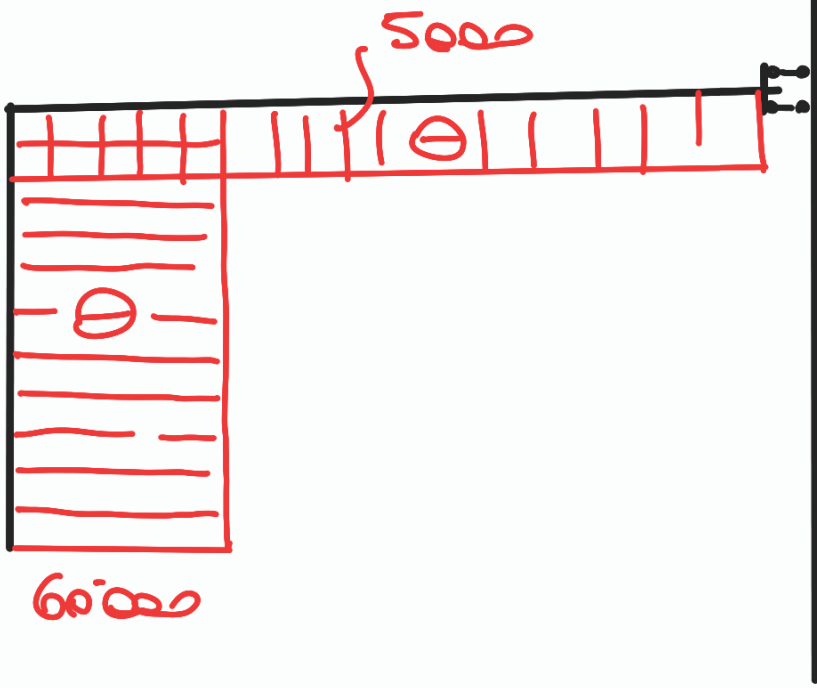
$$H_C = 300000 \text{ Nm}$$

$$M_D = 400000 \text{ Nm}$$

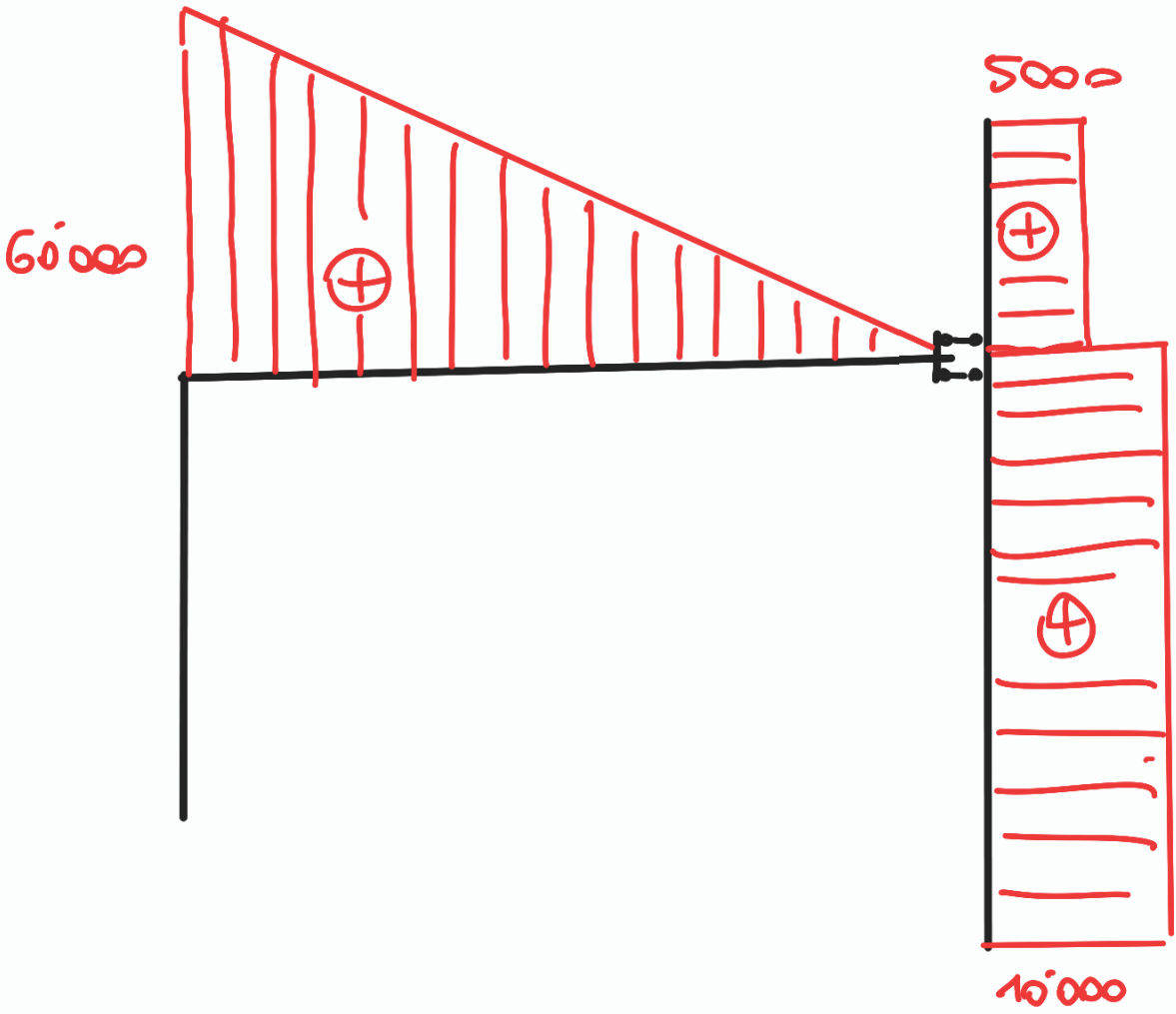
Schema finale delle forze attive e reattive

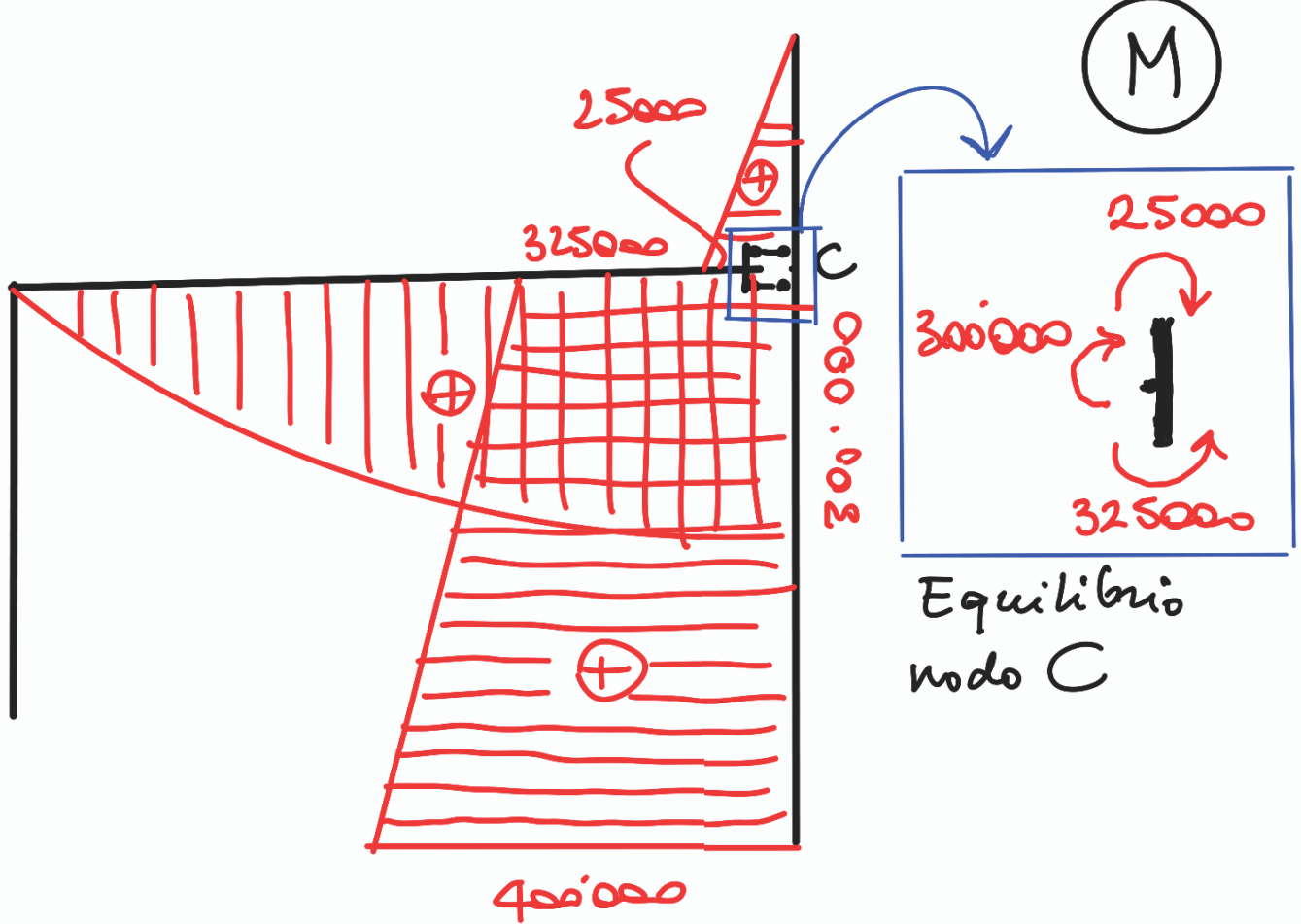


2



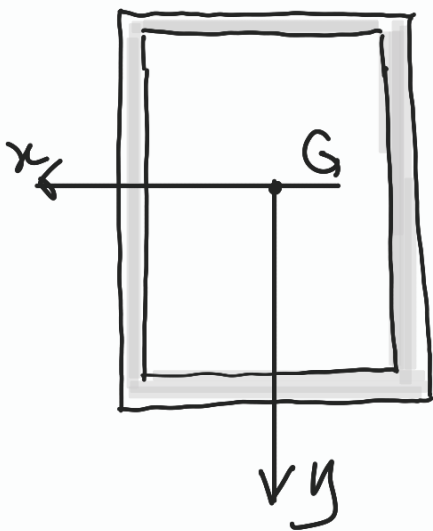
T





2)

● Caratteristiche geometriche della sezione



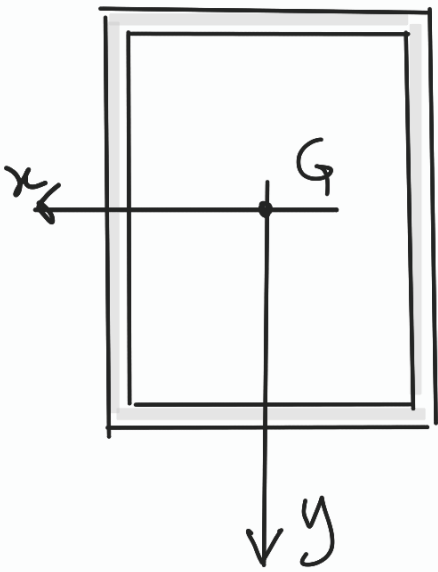
$$A = (0,2 \cdot 2 + 0,2) \cdot 2 \cdot 0,01 = 0,012 \text{ m}^2$$

$$I_x = 2 \cdot \frac{0,2 \cdot 0,01^3}{12} + (0,2 \cdot 0,01 \cdot 0,2^2) \cdot 2$$

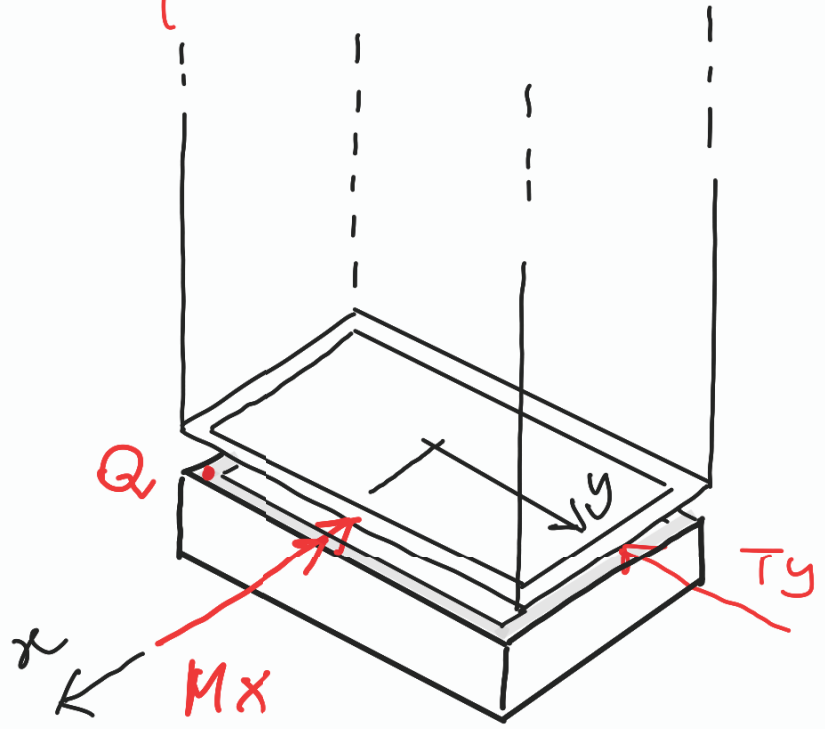
$$+ 2 \cdot \frac{0,01 \cdot 0,4^3}{12} = 3,33 \cdot 10^{-8} + 1,6 \cdot 10^{-4} + 1,06 \cdot 10^{-4} = 2,667 \cdot 10^{-4} \text{ m}^4$$

Il momento di inerzia I_y non è

● Calcolo delle tensioni nelle sez. S-S



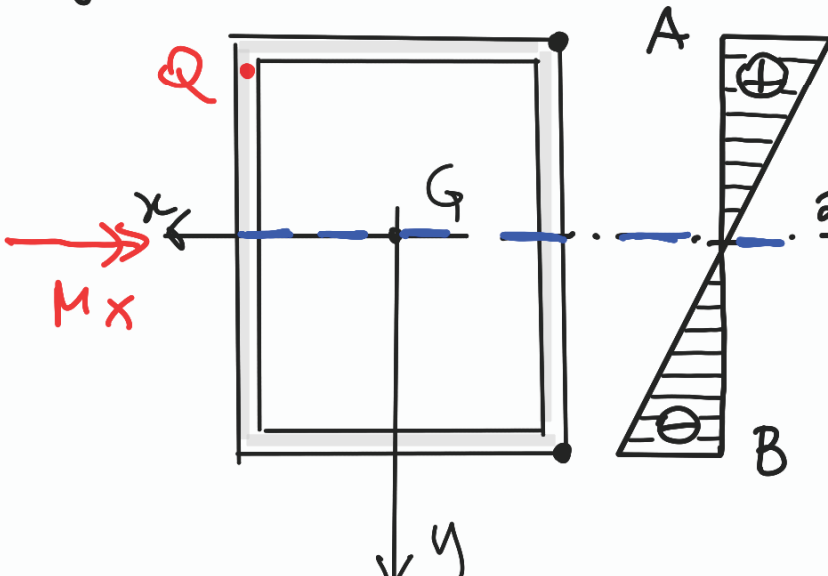
$$\left\{ \begin{array}{l} N = 0 \quad N \\ T = 10000 \quad N \\ M_x = 400000 \quad N \cdot m \end{array} \right.$$



Sez. S-S

(Base del pilastro CD)

● Flessione retta (f. di Navier)
(guardo lo sez d'incastro S dell'alto)



$$\sigma_z(y) = \frac{M_x}{I_x} \cdot y$$

asse neutro $y = 0$

$$\sigma_z(A) = \frac{-400000}{2.667 \cdot 10^{-4}} \cdot (-0,2)$$

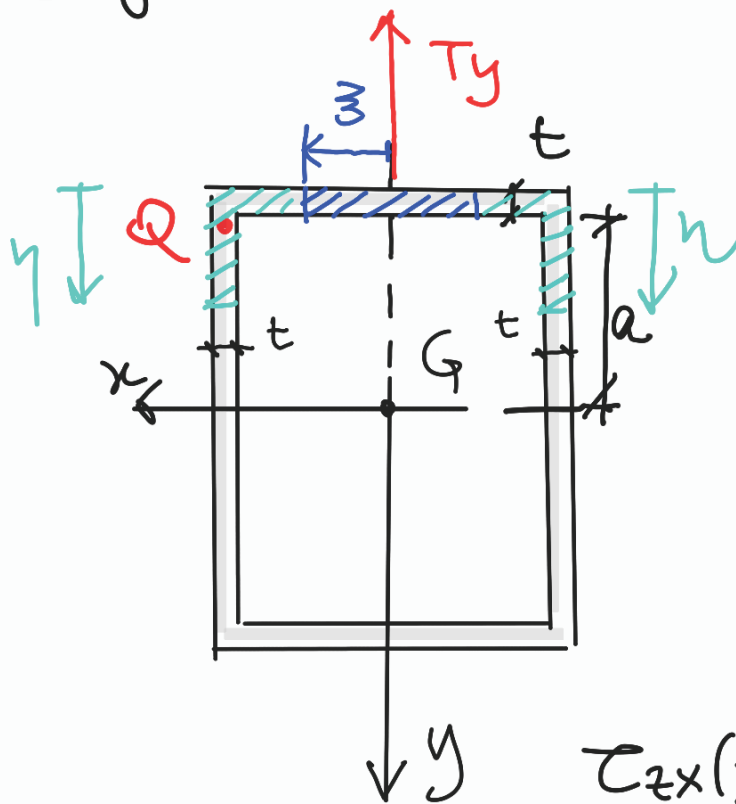
$$= 2,99 \cdot 10^8 \text{ Pa}$$

$$\sigma_z(B) = -2,99 \cdot 10^8 \text{ Pa}$$

$$\sigma_z(Q) \approx +299 \text{ MPa}$$

(il punto Q ha circa la stessa coord. y del punto A)

● Taglio (f. di Jourawski) $\tau_{zx} = - \frac{T_y S_x^*}{I_x \cdot b}$



negativo
↑

$$\tau_{zx}(z) = - \frac{T_y (2 \int t \cdot a)}{I_x \cdot 2t}$$

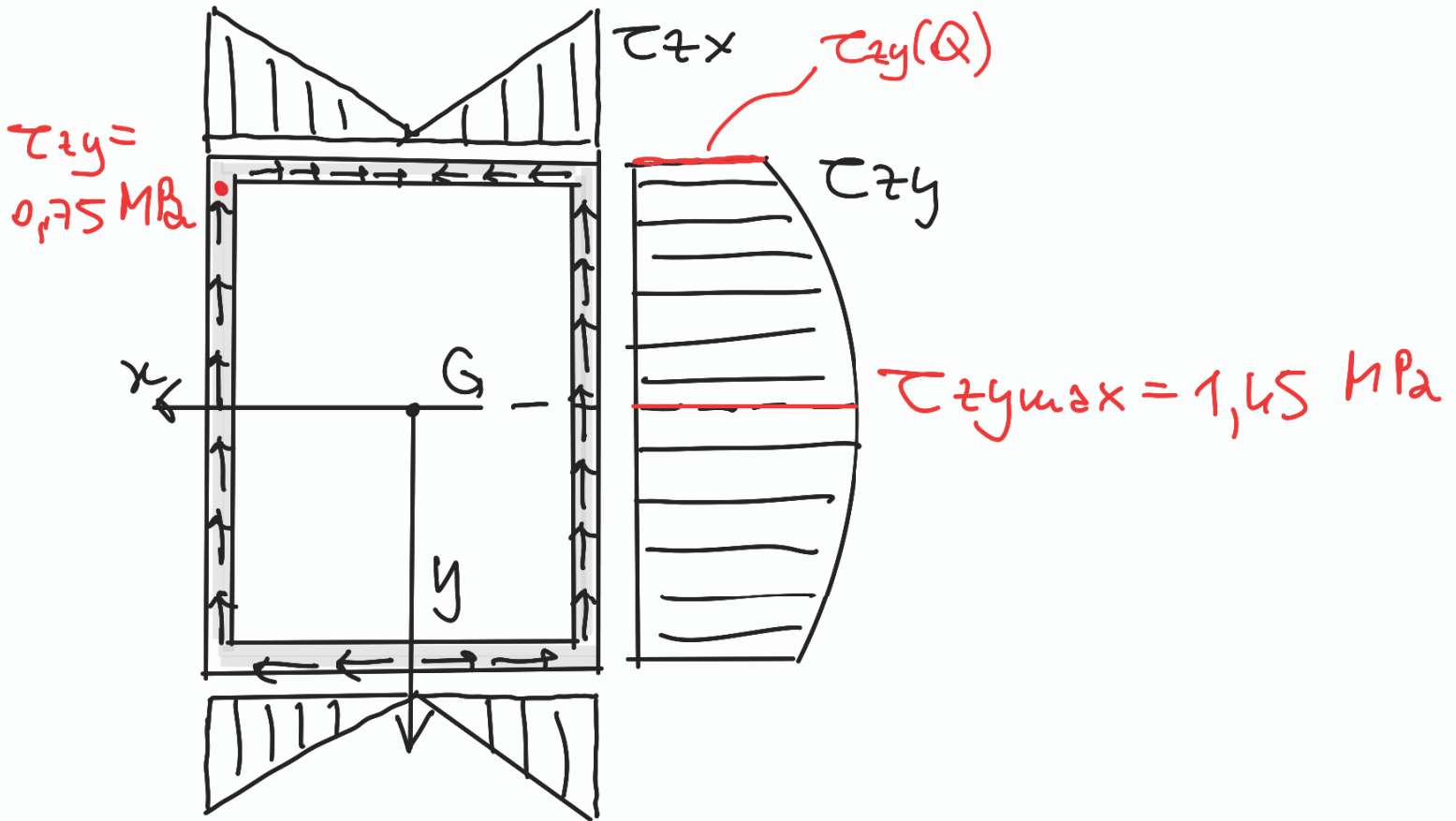
$$\tau_{zx}(z=0,1) = \frac{-10000 (2 \cdot 0,1 \cdot 0,01 \cdot 0,2)}{2,667 \cdot 10^{-4} \cdot 2 \cdot 0,01}$$

$$= -7,50 \cdot 10^5 \text{ Pa}$$

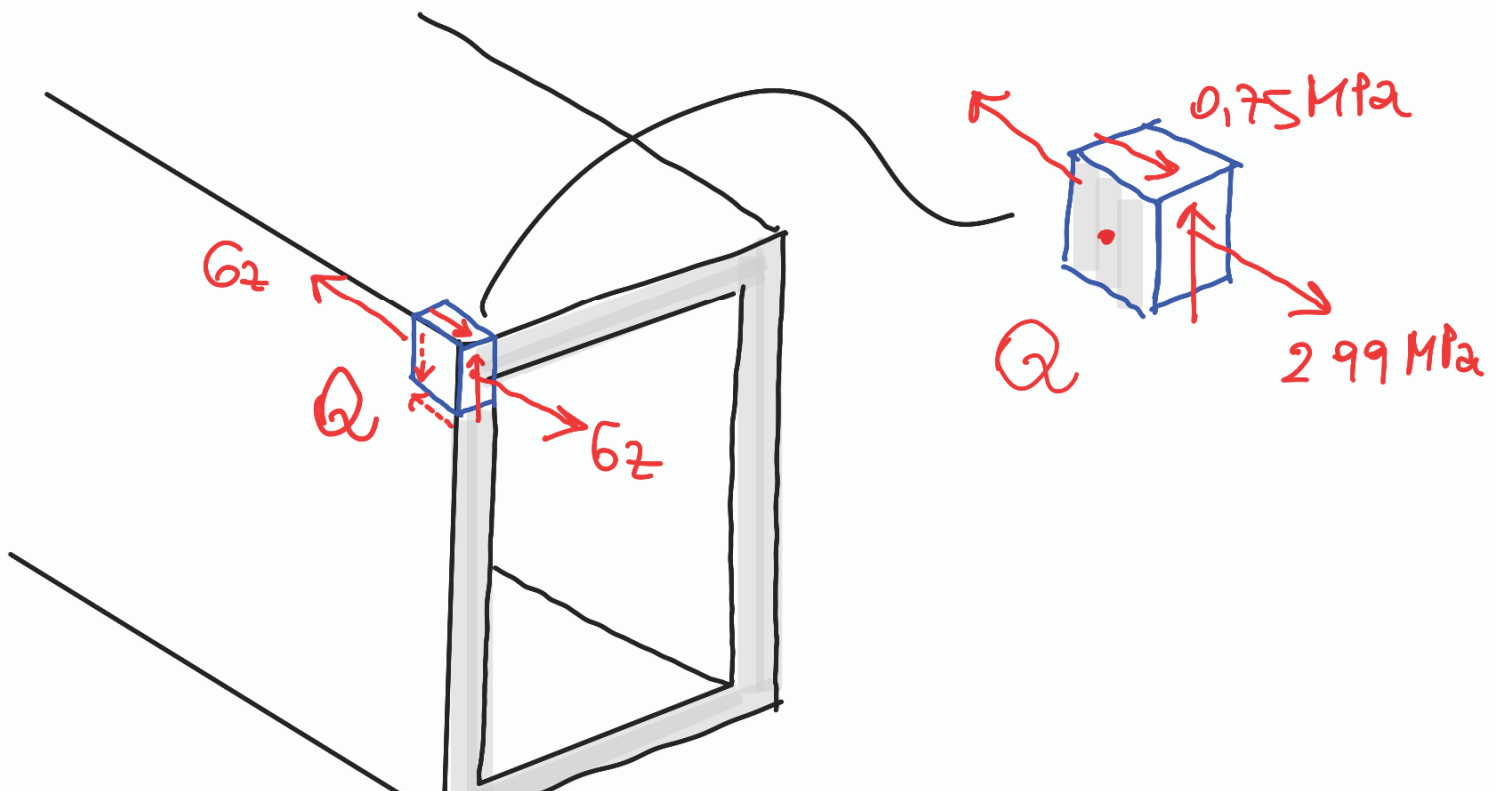
$$\tau_{zx}(z=0,1) = \tau_{zy}(Q) = 7,50 \cdot 10^5 \text{ Pa entrante}$$

$$\tau_{zy}(\eta) = + \frac{10000 \cdot [-0,2 \cdot 0,01 \cdot 0,2 - 2 \cdot 0,01 \cdot \eta (0,1 - \frac{\eta}{2})]}{I_x \cdot t}$$

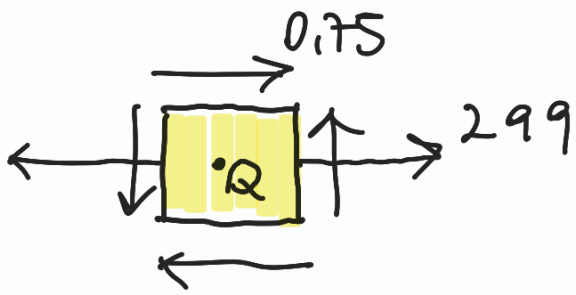
$$\tau_{zy}(y=0,2) = \tau_{zy\max} = \frac{10000}{2.667 \cdot 10^{-4} \cdot 2 \cdot 0,01} = 1,45 \cdot 10^8 \text{ Pa}$$



- Verifica di resistenza nel punto Q delle set S
e cerchio di Mohr



Punto Q (MPa)

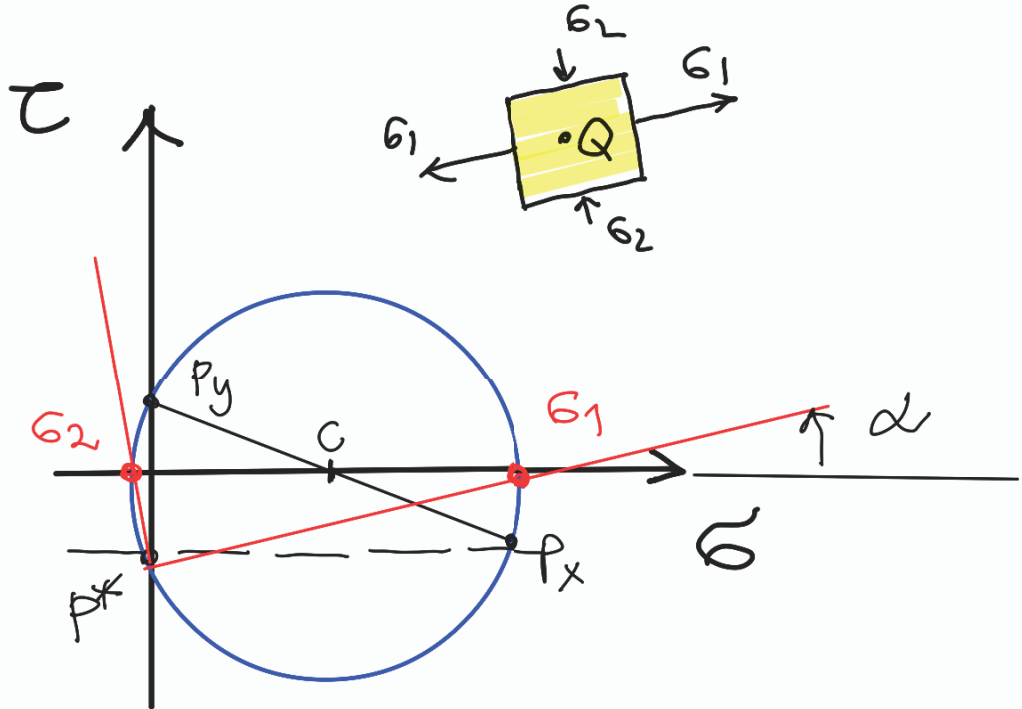


$$P_x = (299 ; -0,75)$$

$$P_y = (0 ; 0,75)$$

Piano di Mohr

(disegno non in scala)



$$x_c = +149,5 \text{ MPa} \quad r = 149,51 \text{ MPa}$$

$$\begin{cases} \sigma_1 = x_c + r = 299,01 \text{ MPa} \\ \sigma_2 = x_c - r = -0,01 \text{ MPa} \end{cases}$$

$$\alpha = \frac{1}{2} \arctg \frac{\tau_{xy}}{\frac{\sigma_x - \sigma_y}{2}} = +0,144^\circ \text{ antiorario}$$

- Verifica di resistenza nel punto

Criterio di Von Mises

$$\sigma_{eq. VM} = \sqrt{\sigma^2 + 3\tau^2}$$

$$\sigma_{eq. VM} = \sqrt{299^2 + 3 \cdot 0,75^2} = 299,01 \text{ MPa} < 300 \text{ MPa}$$

verificato

