

1) • Verifica che la struttura è isostatica

$n_{aste} = 2$

$v = 3 + 2 + 1 = 6$

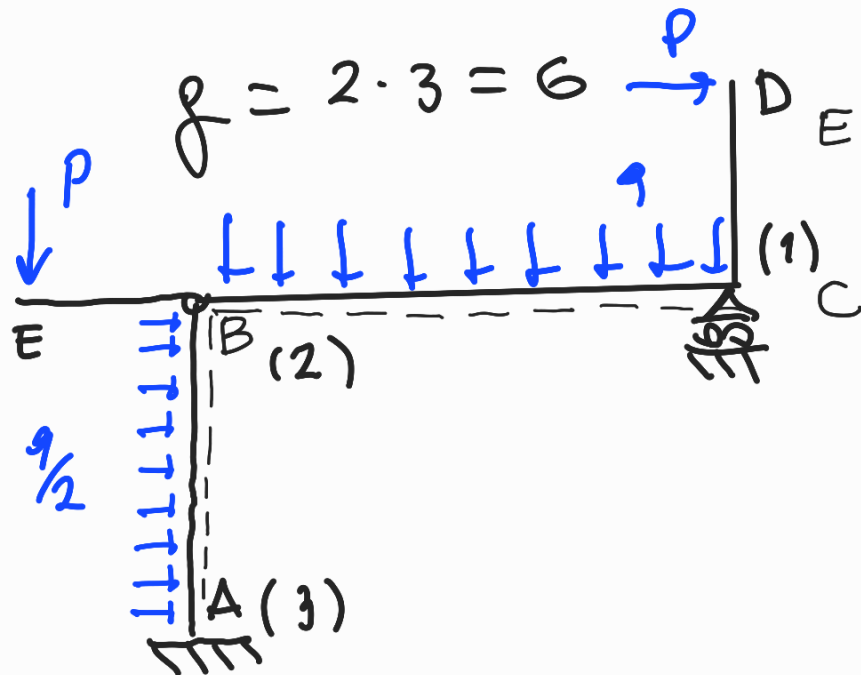
$g = 2 \cdot 3 = 6$

\rightarrow isostatica

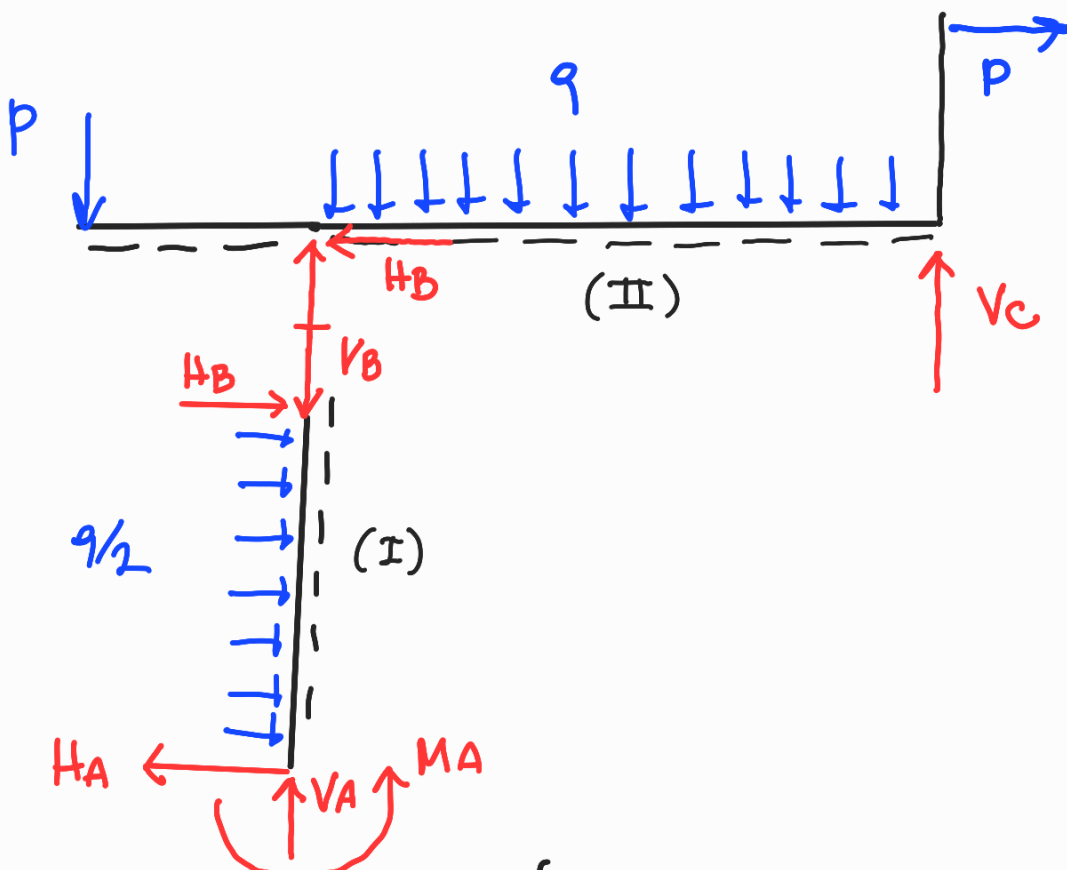
$L = 4 \text{ m}$

$q = 8000 \text{ N/m}$

$P = 10'000 \text{ N}$



• Calcolo delle reazioni vincolari

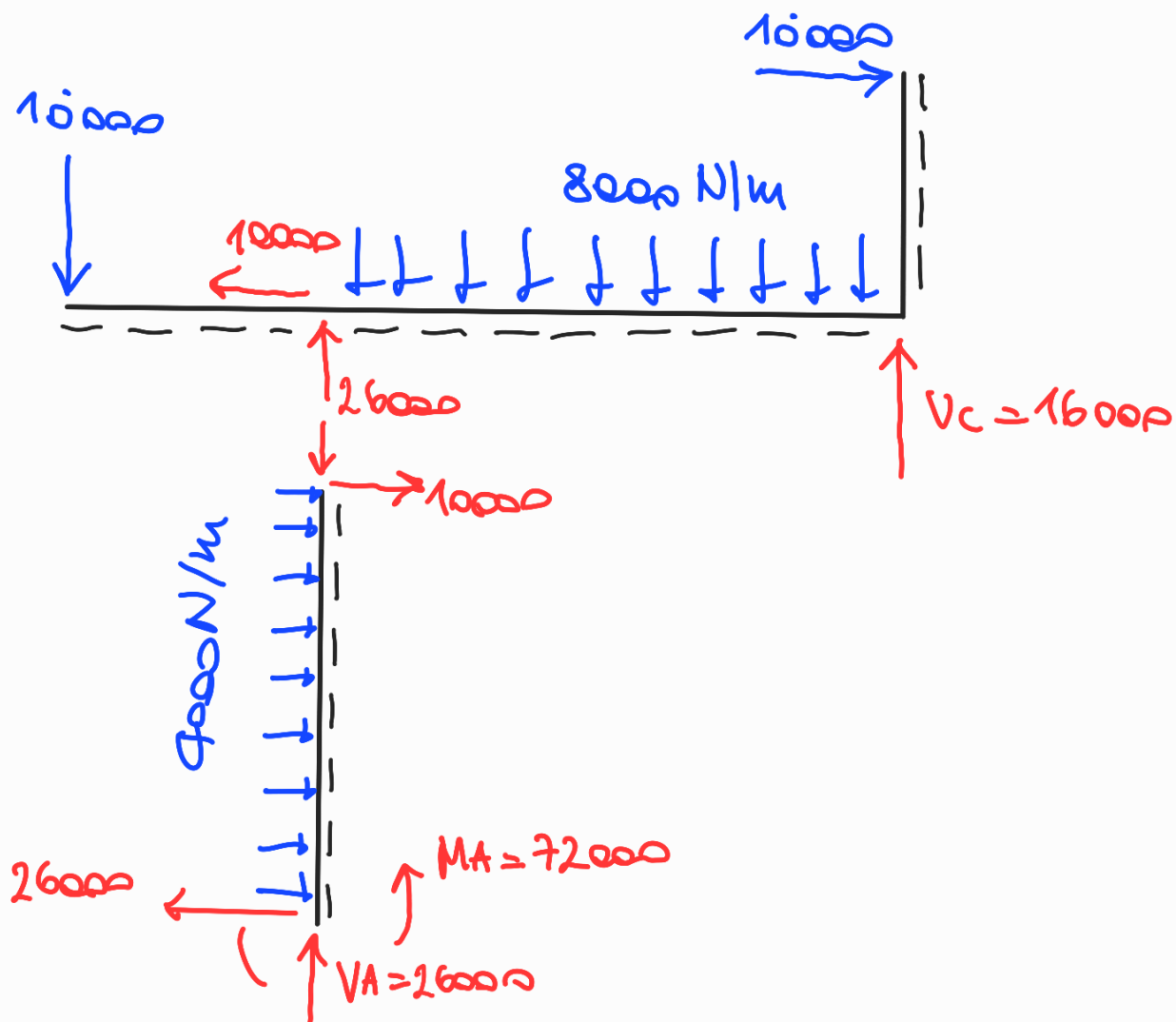


Equilibri delle aste;

$$\begin{cases}
 \rightarrow -H_A + \frac{q}{2} \cdot L + H_B = 0 & H_A = H_B + \frac{q}{2} L = 26000 \text{ N} \\
 \uparrow V_A - V_B = 0 & V_A = 26000 \text{ N} \\
 \curvearrowleft_A - M_A + \frac{q}{2} \frac{L^2}{2} + H_B \cdot L = 0 & M_A = \frac{qL^2}{4} + H_B \cdot L = 72000 \text{ Nm}
 \end{cases}$$

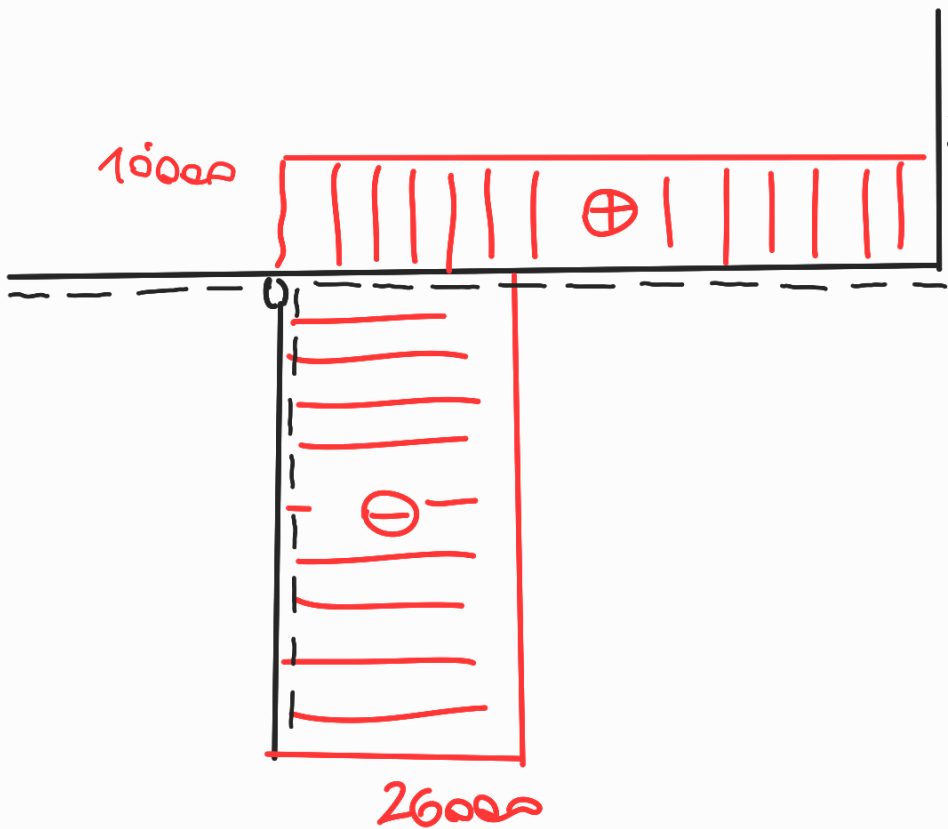
$$\begin{cases}
 \rightarrow -H_B + P = 0 & H_B = P = 10000 \text{ N} \\
 \uparrow V_B - qL + V_C - P = 0 & V_B = P + qL - V_C = 26000 \text{ N} \\
 \curvearrowleft_B - \frac{PL}{2} + \frac{qL^2}{2} - V_C \cdot L + \frac{PL}{2} = 0 & V_C = +\frac{qL}{2} = 16000 \text{ N}
 \end{cases}$$

Schema finale delle forze attive e reattive

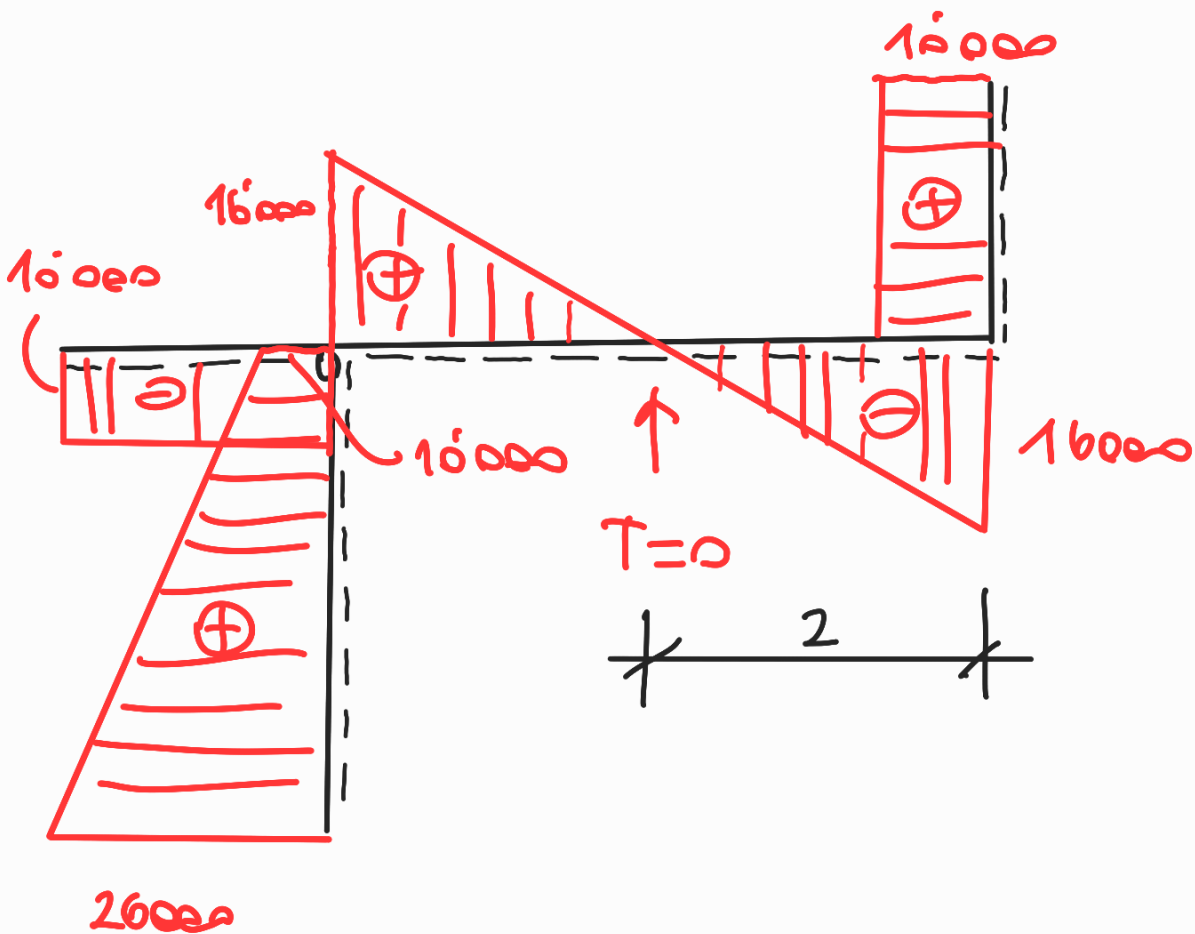


• Diagrammi delle azioni interne

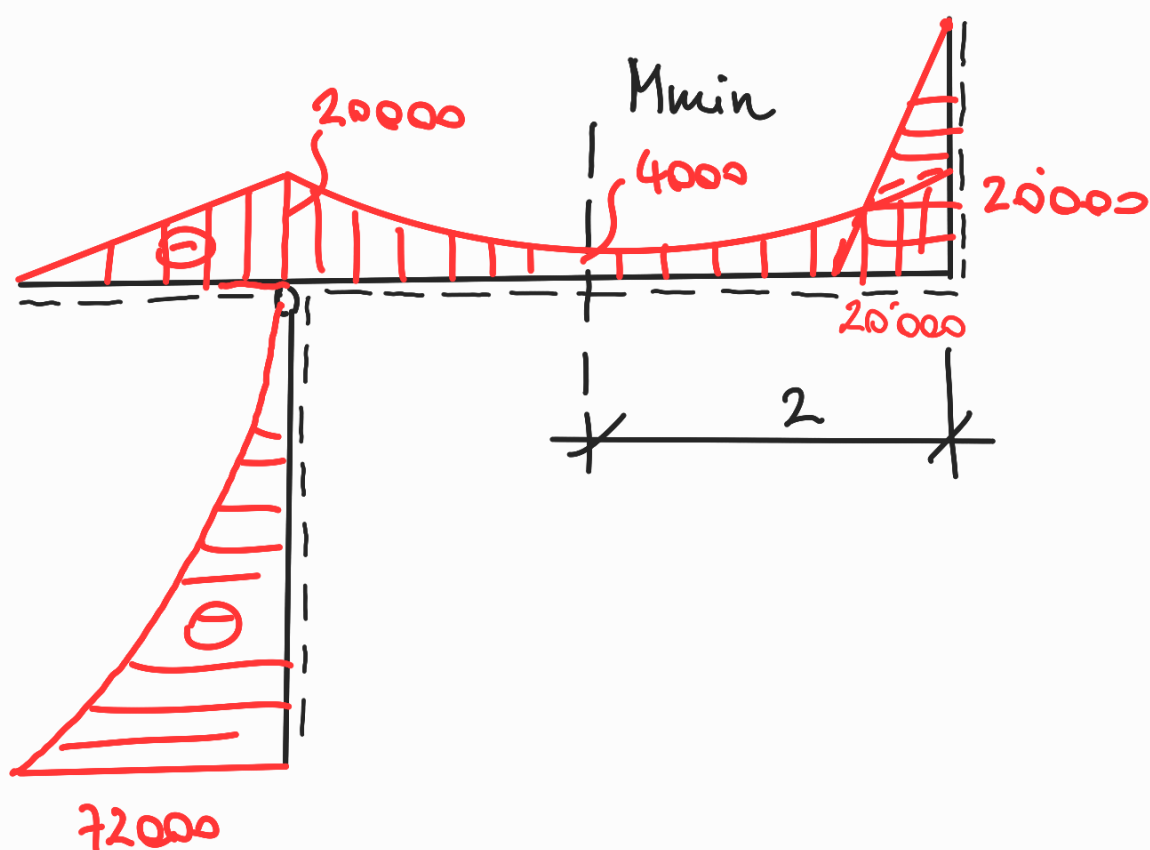
(2)



(T)

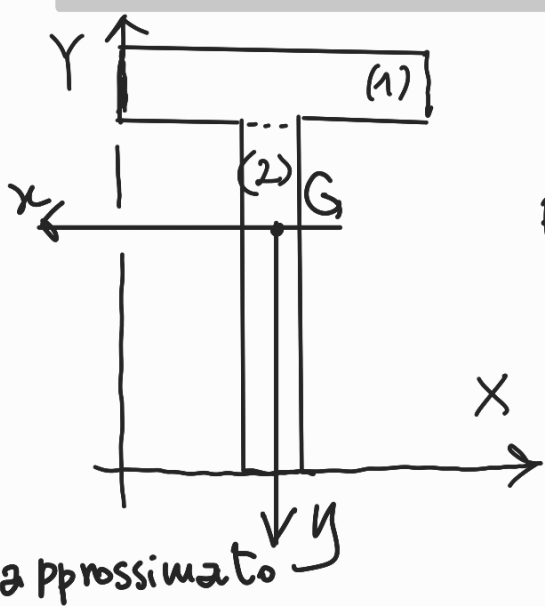


M



2)

• Caratteristiche geometriche della sezione



$$A = 0,2 \cdot 0,02 + 0,4 \cdot 0,01 = 8 \cdot 10^{-3} \text{ m}^2$$

Posizione baricentro

$$X_G = 0,1 \text{ m}$$

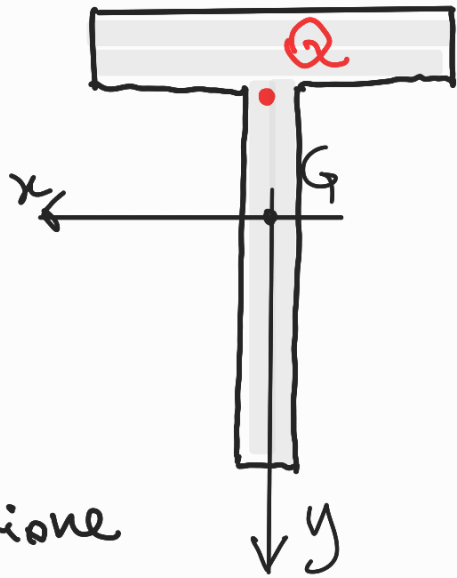
$$Y_G = \frac{S_x}{A} = \frac{0,2 \cdot 0,02 \cdot 0,4 + 0,4 \cdot 0,01 \cdot 0,1}{8 \cdot 10^{-3}} = 0,3 \text{ m}$$

approssimato

$$I_x = \frac{0,2 \cdot 0,02^3}{12} + 0,2 \cdot 0,02 \cdot 0,1^2 + \frac{0,01 \cdot 0,4^3}{12} + 0,4 \cdot 0,01 \cdot 0,1^2 = 1,33 \cdot 10^{-4} \text{ m}^4$$

Il momento di inerzia I_y non è necessario.

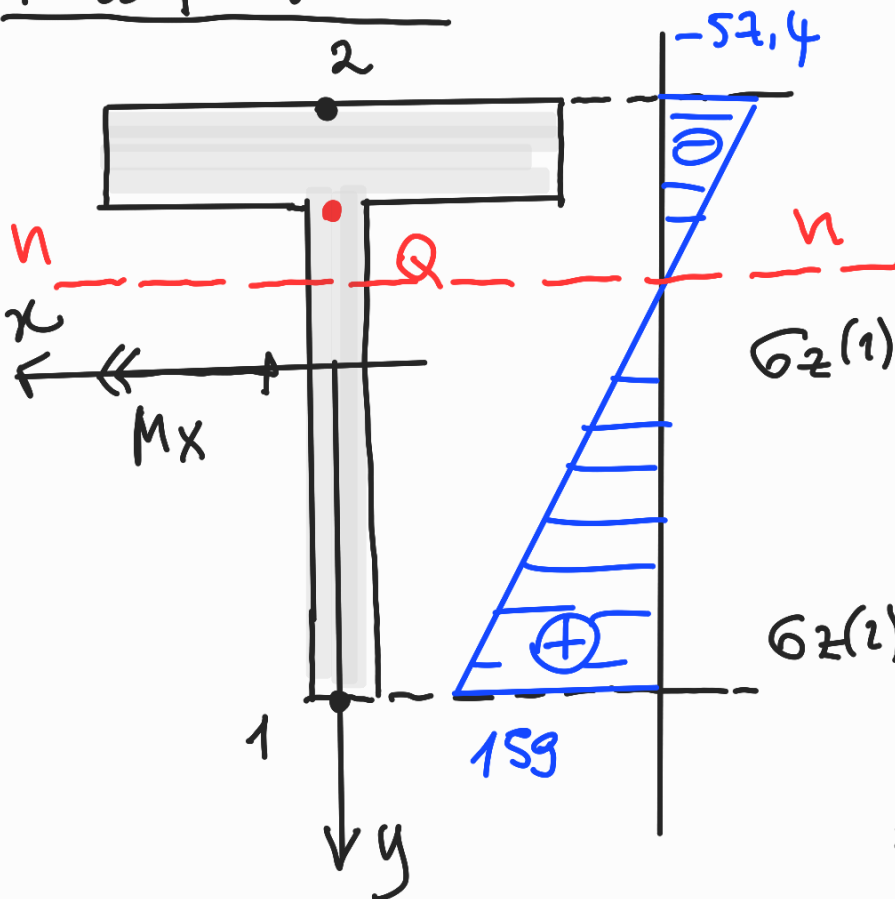
● Calcolo delle Tensioni nelle sez. S-S



$$\left\{ \begin{array}{l} N = -26000 \text{ N} \\ T = 26000 \text{ N} \\ M_x = 72000 \text{ Nm} \end{array} \right.$$

Sezione
posta alla base del
pilastro AB

● Pressoflessione



$$\sigma_z(y) = \frac{N}{A} + \frac{M_x y}{I_x}$$

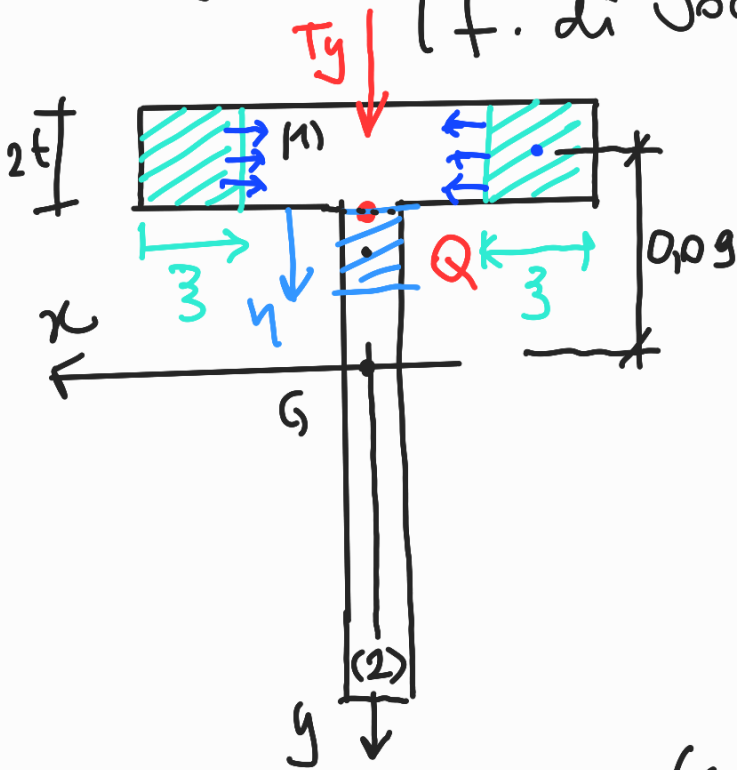
$$\sigma_z(1) = \frac{-26000}{8 \cdot 10^{-3}} + \frac{72000}{1,33 \cdot 10^{-4}} \cdot 0,3 = 159 \text{ MPa}$$

$$\sigma_z(2) = \frac{-26000}{8 \cdot 10^{-3}} + \frac{72000}{1,33 \cdot 10^{-4}} \cdot (-0,1) = -57,4 \text{ MPa}$$

$$\sigma_z(Q) = \frac{-26000}{8 \cdot 10^{-3}} + \frac{72000}{1,33 \cdot 10^{-4}} \cdot (-0,09) = -51,9 \text{ MPa}$$

• Taglio

(f. di Jourawski) $\tau_{zs} = - \frac{T_y S_x^*}{I_x \cdot b}$

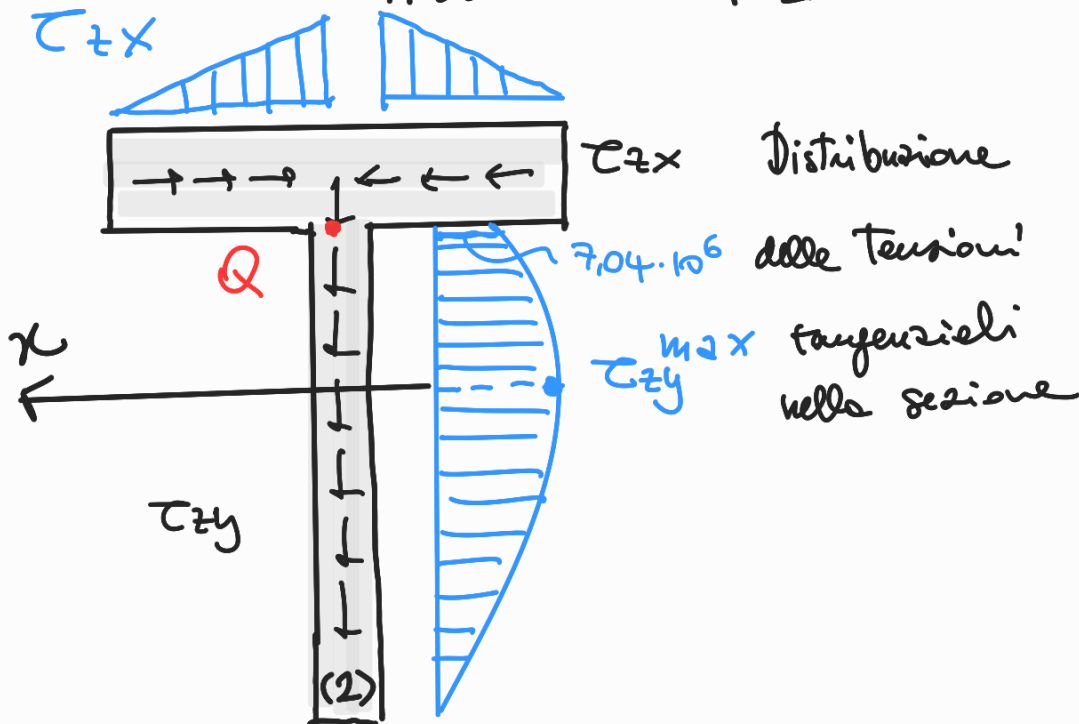


$$\tau_{zx}(z) = - \frac{T_y \cdot (2z \cdot 2t \cdot (-909))}{I_x \cdot 4t}$$

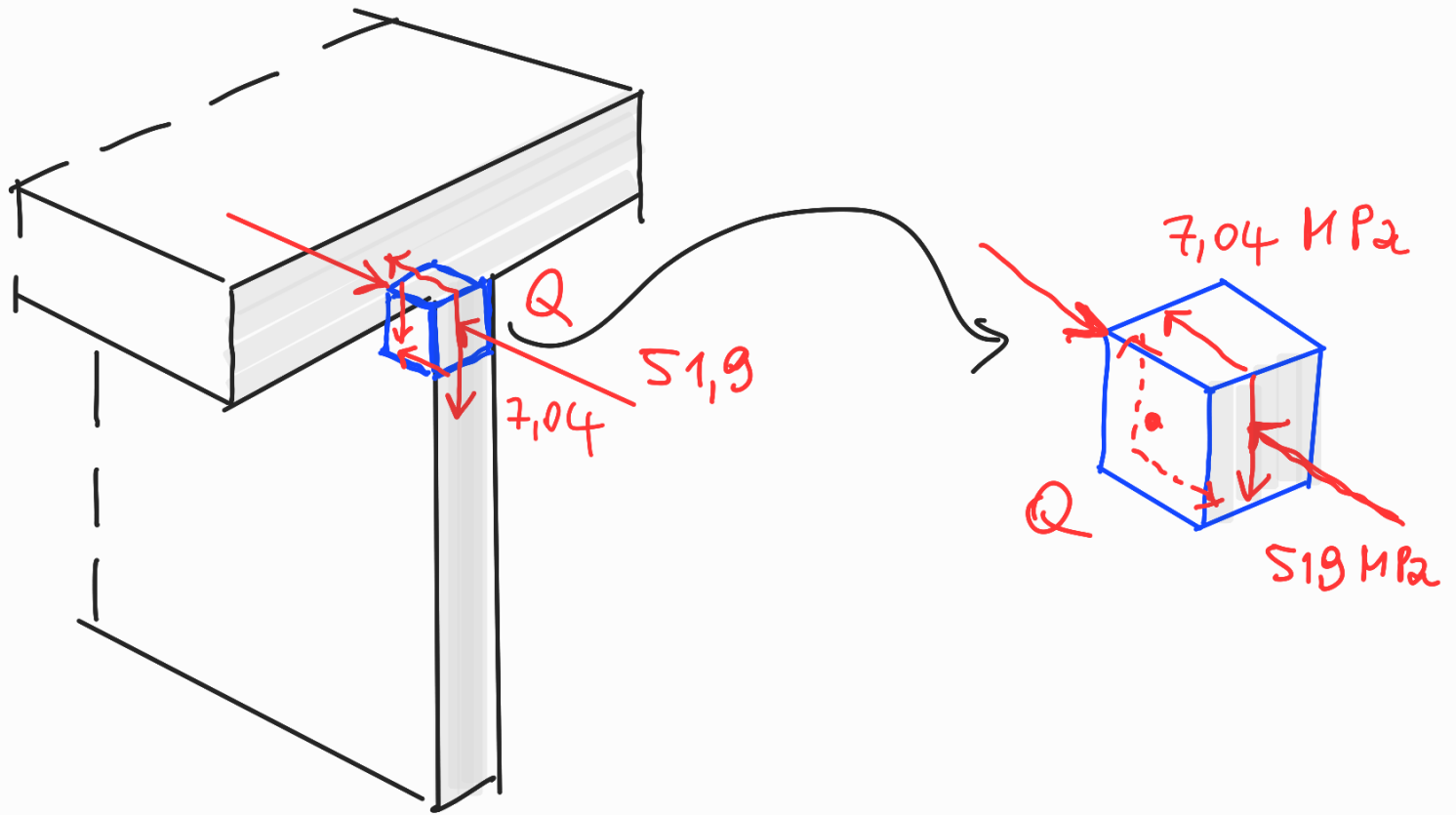
$$\tau_{zx}(z=0,1) = - \frac{26000 \cdot (2 \cdot 0,1 \cdot 0,02 \cdot (-0,09))}{1,33 \cdot 10^{-4} \cdot 0,02 \cdot 2} = -7 \cdot 10^4 \text{ Pa}$$

$\tau_{zx}(Q) = 7,04 \text{ MPa}$

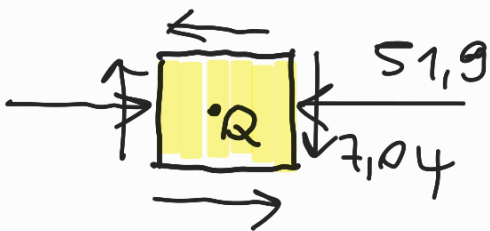
$$\tau_{zy}(\eta) = - \frac{T_y [S_x^{(1)} + \eta \cdot 0,01 \cdot (\frac{\eta}{2} - 0,08)]}{1,33 \cdot 10^{-4} \cdot 0,01}$$



- Verifica di resistenza nel punto Q della set S
e cerchio di Mohr



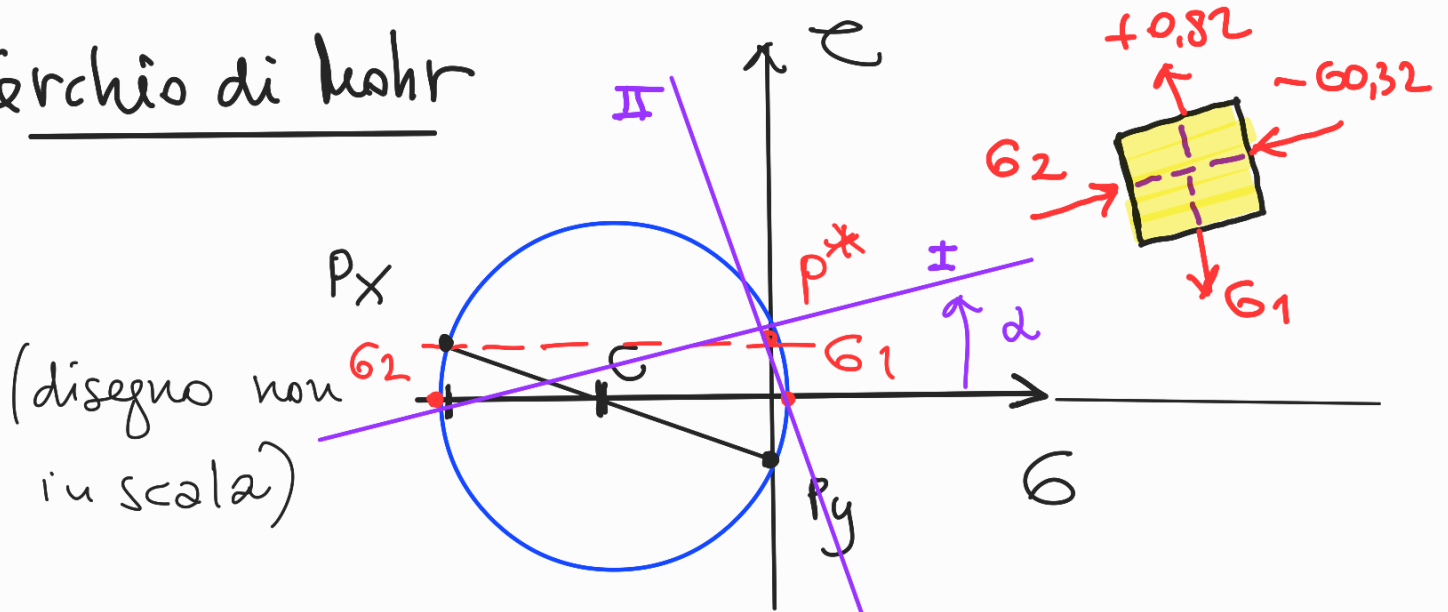
Punto Q (MPa) stato tensionale



$$P_x = (-51,9 ; 7,04)$$

$$P_y = (0 ; -7,04)$$

Cerchio di Mohr



$$\sigma_c = \frac{-59,5 + 0}{2} = -29,75 \text{ MPa}$$

$$r = 30,57 \text{ MPa}$$

$$\begin{cases} \sigma_1 = \sigma_c + r = 0,82 \text{ MPa} & \text{Tensioni} \\ \sigma_2 = \sigma_c - r = -60,32 \text{ MPa} & \text{principali} \end{cases}$$

$$\alpha = \frac{1}{2} \arctan \frac{\tau_{xy}}{\frac{\sigma_x - \sigma_y}{2}} = 6,66^\circ \text{ antiorario}$$

- Verifica di resistenza nel punto Q

Criterio di Tresca

$$\sigma_{eqTR} = \sqrt{\sigma^2 + 4\tau^2}$$

$$G_{eqTR} = \sqrt{59,5^2 + 4 \cdot 7,04^2} = 61,14 \text{ MPa}$$

$$G_{eqTR} = 61,14 \text{ MPa} < 280 \text{ MPa}$$

verificato
